

**CITY OF LAFAYETTE
STORMWATER TECHNICAL
STANDARDS MANUAL**

2023 Version

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Chapter One

INTRODUCTION

This document, the City of Lafayette Stormwater Technical Standards Manual, prepared by Christopher B. Burke Engineering, LLC. for the City of Lafayette, contains the necessary technical standards for administering the requirements of IDEM MS4 GP and the City of Lafayette Stormwater Code. This document should be considered as a companion document to the ordinance. Whereas the ordinance contains the majority of the regulatory authority and general requirements of comprehensive stormwater management, this document contains the necessary means and methods for achieving compliance with the ordinance. It is not intended as a regulatory document, but rather guidance to assist plan reviewers, developers, and designers. In case there are conflicts between the requirements contained in this document and the ordinance, the requirements of the ordinance should prevail.

This document contains formulas and methodologies for the review and design of both stormwater quantity and stormwater quality facilities. Chapters 2 through 6 contain stormwater conveyance and detention calculations and requirements. Chapter 7 contains information on erosion control requirements and other pollution prevention measures for active construction sites. Chapters 8 and 9 cover calculations required to properly size and design stormwater quality features that will treat runoff long-term following construction completion. This includes both Conventional and Low Impact Development (LID) approaches. Chapter 10 contains miscellaneous standards regarding the requirements associated with proposed dams or levees, development downstream of existing dams, no net loss floodplain storage requirements, FEH requirements, Bluff Impact Zone requirements, and special stormwater requirements for solar farms. A comprehensive glossary of terms is provided in Appendix A. Appendix B contains several useful and necessary standard forms. Best Management Practices (BMPs) for erosion control measures during the construction phase are contained in Appendix C. It is the intent of the City of Lafayette that material presented in Appendix C will be revised or eliminated once the Indiana Storm Water Quality Manual (ISWQM) is updated to include details regarding the BMPs currently included in Appendix C. Appendix D contains non-structural and structural post-construction BMP Fact Sheets as well as Recommended Plant Lists, Recommended Materials, Soil Infiltration Testing Protocol, BMP Maintenance Checklists, and a Maintenance Agreement for post-construction BMPs.

The site designer is encouraged to review the LID discussion in Chapter 8 prior to the site design to take advantage of runoff reduction recognitions provided towards water quantity calculations if LID practices are utilized as part of the site design.



Chapter Two

METHODOLOGY FOR DETERMINATION OF RUNOFF RATES

Runoff rates shall be computed for the area of the parcel under development plus the area of the watershed flowing into the parcel under development. The rate of runoff which is generated as the result of a given rainfall intensity may be calculated as follows:

A. Development Sites Less than or Equal to 5 Acres in Size, With a Contributing Drainage Area Less than or Equal to 50 Acres and No Depressional Storage

The Rational Method may be used. A computer model, such as TR-55 (NRCS), TR-20 (NRCS), HEC-HMS (COE), and HEC-1 (COE), that can generate hydrographs based on the NRCS TR-55 time of concentration and curve number calculation methodologies may also be used with the duration storm that produces the highest peak runoff rate. Note that for the purpose of determining the post-developed conditions curve numbers, due to significant disturbance to the upper soil layers during the construction activities, the initially determined hydrologic soil group for disturbed areas should be changed to the next less infiltrating capacity category (i.e., A to B, B to C, and C to D).

LID Exception: If Low Impact Development (LID) approach is pursued in satisfying the requirements noted in Chapter 8 (Post-Construction Water Quality BMPs), the post-developed CN for the protected undisturbed or restored disturbed areas meeting the requirements described in Chapter 8 and BMP fact sheets may be determined based on pre-development underlying soil layer.

In the Rational Method, the peak rate of runoff, Q , in cubic feet per second (cfs) is computed as:

$$Q = CIA$$

Where: C = Runoff coefficient, representing the characteristics of the drainage area and defined as the ratio of runoff to rainfall.

I = Average intensity of rainfall in inches per hour for a duration equal to the time of concentration (t_c) for a selected rainfall frequency.

A = Tributary drainage area in acres.

Values for the runoff coefficient "C" are provided in **Tables 2-1** and **2-2**, which show values for different types of surfaces and local soil characteristics. The composite "C" value used for a given drainage area with various surface types shall be the weighted average value for the total area calculated from a breakdown of individual areas having different surface types. **Table 2-3** provides runoff coefficients and inlet times for different land use classifications.

Rainfall intensity shall be determined from the rainfall frequency data shown in **Table 2-4**.

In general, the time of concentration (t_c) methodology to be used for all stormwater management projects within City of Lafayette shall be as outlined in the U.S. Department of Agriculture (USDA) - NRCS TR-55 Manual. In urban or developed areas, the methodology to be used shall be the sum of the inlet time and flow time in the stormwater facility from the most remote part of the drainage area to the point under consideration. The flow time in the storm sewers may be estimated by the distance in feet divided by velocity of flow in feet per second. The velocity shall be determined by the Manning's Equation (see Chapter 4). Inlet time is the combined time required for the runoff to reach the inlet of the storm sewer. It includes overland flow time and flow time through established surface drainage channels such as swales, ditches, and sheet flow across such areas as lawns, fields, and other graded surfaces.

**TABLE 2-1
Urban Runoff Coefficients**

<i>Type of Surface</i>	<i>Runoff Coefficient "C"</i>
Hard Surfaces	
Asphalt	0.82
Concrete	0.85
Roof	0.85
Gravel/Stone	0.50
Lawns (Sandy)	
Flat (0-2% Slope)	0.07
Rolling (2-7% Slope)	0.12
Steep (Greater than 7% Slope)	0.17
Lawns (Clay)	
Flat (0-2% Slope)	0.16
Rolling (2-7% Slope)	0.21
Steep (Greater than 7% Slope)	0.30

Source: LTAP Stormwater Drainage Manual, July 1995.

The coefficients of this tabulation are applicable to storms of 1 to 10 year frequencies. Coefficients for less frequent, higher intensity storms shall be modified as follows*:

<u>Return Period (yrs.)</u>	<u>Multiply (C) by</u>
25	1.10
50	1.20
100	1.25

* The upper limit for runoff is 1.0.

TABLE 2-2

Rural Runoff Coefficients

Type of Surface	Runoff Coefficient "C"
Woodland (Sandy)	
Flat (0-5% Slope)	0.10
Rolling (5-10% Slope)	0.25
Steep (Greater than 10% Slope)	0.30
Woodland (Clay)	
Flat (0-5% Slope)	0.30
Rolling (5-10% Slope)	0.35
Steep (Greater than 10% Slope)	0.50
Pasture (Sandy)	
Flat (0-5% Slope)	0.10
Rolling (5-10% Slope)	0.16
Steep (Greater than 10% Slope)	0.22
Pasture (Clay)	
Flat (0-5% Slope)	0.30
Rolling (5-10% Slope)	0.36
Steep (Greater than 10% Slope)	0.42
Cultivated (Sandy)	
Flat (0-5% Slope)	0.30
Rolling (5-10% Slope)	0.40
Steep (Greater than 10% Slope)	0.52
Cultivated (Clay)	
Flat (0-5% Slope)	0.50
Rolling (5-10% Slope)	0.60
Steep (Greater than 10% Slope)	0.72

Source: LTAP Stormwater Drainage Manual, July 1995.

The coefficients of this tabulation are applicable to storms of 1 to 10 year frequencies. Coefficients for less frequent, higher intensity storms shall be modified as follows*:

<u>Return Period (yrs.)</u>	<u>Multiply (C) by</u>
25	1.10
50	1.20
100	1.25

* The upper limit for runoff is 1.0.

**TABLE 2-3
Runoff Coefficients "C" by Land Use and Typical Inlet Times**

Land Use	Runoff Coefficients			Inlet Times (Minutes) ⁴
	Flat ¹	Rolling ²	Steep ³	
Commercial (CBD)	0.75	0.83	0.91	5
Commercial (Neighborhood)	0.54	0.60	0.66	5-10
Industrial	0.63	0.70	0.77	
Garden Apartments	0.54	0.60	0.66	
Churches	0.54	0.60	0.66	
Schools	0.31	0.35	0.39	10-15
Semi Detached Residential	0.45	0.50	0.55	
Detached Residential	0.40	0.45	0.50	
Quarter Acre Lots	0.36	0.40	0.44	
Half Acre Lots	0.31	0.35	0.39	
Parkland	0.18	0.20	0.22	To be Computed

Source: LTAP Stormwater Drainage Manual, July 1995.

- ¹ Flat terrain involves slopes of 0-2%.
- ² Rolling terrain involves slopes of 2-7%.
- ³ Steep terrain involves slopes greater than 7%.
- ⁴ Interpolation, extrapolation and adjustment for local conditions shall be based on engineering experience and judgment.

The coefficients of this tabulation are applicable to storms of 1 to 10 year frequencies. Coefficients for less frequent, higher intensity storms shall be modified as follows*:

Return Period (yrs.)	Multiply (C) by
25	1.10
50	1.20
100	1.25

* The upper limit for runoff is 1.0.

B. Development Sites Greater Than 5 Acres in Size or Contributing Drainage Area Greater than 50 Acres or With Significant Depressional Storage

The runoff rate for these development sites and contributing drainage areas shall be determined by a computer model that can generate hydrographs based on the NRCS TR-55 time of concentration and curve number calculation methodologies and the Huff Third Quartile (50%) Rainfall Distribution. Note that for the purpose of determining the post-developed conditions curve numbers, due to significant disturbance to the upper soil layers during the construction activities, the initially determined hydrologic soil group for disturbed areas should be changed to the next less infiltrating capacity category (i.e., A to B, B to C, and C to D).

LID Exception: If Low Impact Development (LID) approach is pursued in satisfying the requirements noted in Chapter 8 (Post-Construction Water Quality BMPs), the post-developed CN for the protected undisturbed or restored disturbed areas meeting the requirements described in Chapter 8 and BMP fact sheets may be determined based on pre-development underlying soil layer.

A critical duration analysis should be performed to determine which rainfall duration would produce the highest peak runoff rate. Rainfall depth for various frequencies and durations shall be taken from **Table 2-5**. The Huff Third Quartile (50%) distribution ordinates are found in **Table 2-6**. Examples of computer models that can generate such hydrographs include TR-20 (NRCS), HEC-HMS (COE), and HEC-1 (COE). Other models may be acceptable and should be approved by the City of Lafayette prior to their utilization. Examples of other computer models that have been previously approved by the City of Lafayette include PondPack, AdICPR, and XP-SWMM.

TABLE 2-4
Rainfall Intensities for Various Return Periods and Storm Durations (Inches/Hour)

Duration	Return Period (Years)						
	1	2	5	10	25	50	100
5 Min.	4.76	5.65	6.72	7.6	8.7	9.6	10.45
10 Min.	3.7	4.42	5.22	5.86	6.65	7.27	7.87
15 Min.	3.02	3.6	4.27	4.81	5.48	6	6.52
30 Min.	2	2.41	2.92	3.34	3.87	4.29	4.71
1 Hr.	1.22	1.48	1.83	2.12	2.51	2.83	3.15
2 Hrs.	0.71	0.86	1.07	1.25	1.5	1.7	1.91
3 Hrs.	0.51	0.61	0.77	0.9	1.08	1.24	1.4
6 Hrs.	0.3	0.36	0.45	0.53	0.64	0.73	0.83
12 Hrs.	0.17	0.21	0.26	0.3	0.36	0.41	0.46
24 Hrs.	0.1	0.12	0.15	0.17	0.2	0.23	0.25

Source: NOAA, National Weather Service, "Precipitation-Frequency Atlas of the United States", NOAA Atlas14, Volume 2, Version 2, 2004, for Lafayette, Indiana. (Values for intermediate durations can be logarithmically interpolated.)

TABLE 2-5
Rainfall Depths for Various Return Periods and Storm Durations (Inches)

Duration	Return Period (Years)						
	1	2	5	10	25	50	100
5 Min.	0.4	0.47	0.56	0.63	0.72	0.8	0.87
10 Min.	0.62	0.74	0.87	0.98	1.11	1.21	1.31
15 Min.	0.76	0.9	1.07	1.2	1.37	1.5	1.63
30 Min.	1	1.2	1.46	1.67	1.93	2.15	2.35
1 Hr.	1.22	1.48	1.83	2.12	2.51	2.83	3.15
2 Hrs.	1.42	1.71	2.15	2.51	3	3.4	3.83
3 Hrs.	1.53	1.84	2.31	2.7	3.25	3.71	4.2
6 Hrs.	1.8	2.17	2.72	3.18	3.83	4.38	4.96
12 Hrs.	2.1	2.53	3.14	3.65	4.37	4.96	5.58
24 Hrs.	2.38	2.86	3.53	4.07	4.81	5.41	6.02

Source: NOAA, National Weather Service, "Precipitation-Frequency Atlas of the United States", NOAA Atlas 14, Volume 2, Version 2, 2004, for Lafayette, Indiana.

TABLE 2-6
Huff Third Quartile (50%) Rainfall Distribution Ordinates

Cumulative Percent of Storm Time	Cumulative Percent of Storm Depth
0	0
5	3
10	6
15	9
20	12
25	15
30	19
35	23
40	27
45	32
50	38
55	45
60	57
65	70
70	79
75	85
80	89
85	92
90	95
95	97
100	100

C. Development Sites with Drainage Areas Greater than or Equal to One Square Mile

For the design of any major drainage system, as defined in **Appendix A**, the discharge must be obtained from, or be approved by, the IDNR. Other portions of the site must use the discharge methodology in the applicable section of this Article.



Chapter Three

METHODOLOGY FOR DETERMINATION OF DETENTION STORAGE VOLUMES

A. Development Sites Less than or Equal to 5 Acres in Size, With a Contributing Drainage Area Less than or Equal to 50 Acres and No Depressional Storage

The required volume of stormwater storage may be calculated using the Rational Method and based on the runoff from 10-year and 100-year return period storms. A computer model, such as TR-20 (NRCS), HEC-HMS (COE), and HEC-1 (COE), that can generate hydrographs based on the NRCS TR-55 time of concentration and curve number calculation methodologies may also be used along with the duration storm that produces the highest peak storage volume and pond elevation. Note that for the purpose of determining the post-developed conditions curve numbers, due to significant disturbance to the upper soil layers during the construction activities, the initially determined hydrologic soil group for disturbed areas should be changed to the next less infiltrating capacity category (i.e., A to B, B to C, and C to D).

LID Exception: If Low Impact Development (LID) approach is pursued in satisfying the requirements noted in Chapter 8 (Post-Construction Water Quality BMPs), the post-developed CN for the protected undisturbed or restored disturbed areas meeting the requirements described in Chapter 8 and BMP fact sheets may be determined based on pre-development underlying soil layer.

The following 9-step procedure, based on the Rational Method, may be used to determine the required volume of storage.

1. Determine the parcel area in acres "A" tributary to each outlet.
2. Compute 100-year frequency allowable release rate (Q_u) to each outlet based on methodologies provided in Chapter 6 of these technical standards
3. Determine composite runoff coefficient " C_d " based on developed conditions and a 100-year return period. As noted on Tables 2-1 through 2-3, the composite runoff coefficient for the 100-year return period should be multiplied by 1.25.
4. Determine 100-year return rainfall intensity " I_d " for various storm durations " t_d " up through 24 hours for the developed area using **Table 2-4**.
5. Determine developed inflow rates " Q_d " for various storm durations " t_d ", measured in hours.

$$Q_d = (C_d)(I_d)(A_d)$$

6. Compute a storage rate "S(t_d)" for various storm durations "t_d" up through 24 hours for the developed area.

$$S(t_d) = (Q_d) - (Q_u)$$

7. Compute required storage volume "S_R" in acre-feet for each storm duration "t_d". This assumes a triangular hydrograph of duration (2t_d) hours with a peak flow of S(t_d) at t_d hours.

$$S_R = S(t_d) \left(\frac{t_d}{12} \right)$$

8. Select largest storage volume computed in Step 7 for any storm duration "t_d" for detention basin design.
9. Repeat Steps 2 – 8 of this process for the post-developed 10-year storm.

B. Development Sites Greater Than 5 Acres in Size or Contributing Drainage Area Greater than 50 Acres or With Significant Depressional Storage

All runoff detention storage calculations for these development sites shall be prepared using a computer model that can generate hydrographs based on the NRCS TR-55 time of concentration and curve number calculation methodologies. Note that for the purpose of determining the post-developed conditions curve numbers, due to significant disturbance to the upper soil layers during the construction activities, the initially determined hydrologic soil group for disturbed areas should be changed to the next less infiltrating capacity category (i.e., A to B, B to C, and C to D).

LID Exception: If Low Impact Development (LID) approach is pursued in satisfying the requirements noted in Chapter 8 (Post-Construction Water Quality BMPs), the post-developed CN for the protected undisturbed or restored disturbed areas meeting the requirements described in Chapter 8 and BMP fact sheets may be determined based on pre-development underlying soil layer.

The Huff Third Quartile (50%) Rainfall Distribution shall be utilized to determine the required storage volume. A critical duration analysis should be performed to determine which rainfall duration would produce the highest storage volume and pond elevation. The allowable release rates shall be determined based on methodologies provided in Chapter 6 of these technical standards. Examples of computer models that can generate such hydrographs include TR-20 (NRCS), HEC-HMS (COE), and HEC-1

(COE). Other models may be acceptable and should be approved by the City of Lafayette prior to their utilization. Examples of other computer models that have been previously approved by the City of Lafayette include PondPack, AdICPR, and XP-SWMM.



Chapter Four

STORM SEWER DESIGN STANDARDS AND SPECIFICATIONS

All storm sewers, whether private or public, and whether constructed on private or public property shall conform to the design standards and other requirements contained herein. Proposed storm sewer systems must be sized and designed to convey at least the 10-year frequency on-site stormwater runoff, as well as the anticipated 10-year frequency tributary off-site stormwater runoff based on the future developed condition (using Table 6-1 of the Technical Standards). An analysis of the emergency routing of stormwater runoff through the subject development must be provided to confirm that the development will not obstruct the free flow of floodwaters from the tributary off-site property in its current condition and after development. In addition, the Drainage System Overflow Design must be completed in accordance with Section M of this chapter to ensure the safe routing of flood waters through the subject development with the tributary off-site property in its current condition and after development.

A. Design Storm Frequencies

1. All storm sewers, inlets, catch basins and street gutters shall accommodate (subject to the “allowable spread” provisions discussed later in this Section), as a minimum, peak runoff from a 10-year return frequency storm. Additional discharges to storm sewer systems allowed in Section L below of this Section must be considered in all design calculations. For Rational Method analysis, the duration shall be equal to the time of concentration for the drainage area. In computer based analysis, the duration is as noted in the applicable methodology associated with the computer program.
2. Culverts shall be capable of accommodating peak runoff from the 50-year frequency storm of critical duration when crossing under a road which is part of the INDOT Rural Functional Classification System or is classified as interstate highway, divided primary arterial, arterial, and/or collectors by the Tippecanoe County Thoroughfare Plan or provides the only access to and from any portion of any commercial or residential developments. Driveway culvert capacities shall be capacities required for the street classification to which the driveway connects. Greater culvert capacity shall be required to protect the finished floor elevation of buildings from the post-developed 100-year frequency storm when, in the opinion of the design engineer or the City of Lafayette, the finished floor elevation is threatened.

3. For portions of the system considered minor drainage systems, the allowable spread of water on Collector Streets is limited to maintaining two clear 10-foot moving lanes of traffic. One lane is to be maintained on Local Roads, while other access lanes (such as a subdivision cul-de-sac) can have a water spread equal to one-half of their total width.
4. To ensure access to buildings and allow the use of the roadway by emergency vehicles during storms larger than the design storm, an overflow channel from sag inlets to the overflow channel or basin shall be provided at sag inlets so that the maximum depth of water that might be ponded in the street sag shall not exceed 7 inches.
5. Facilities functioning as a major drainage system as defined in **Appendix A** must also meet IDNR design standards.

B. Manning's Equation

Determination of hydraulic capacity for storm sewers sized by the Rational Method analysis must be done using Manning's Equation, where:

$$V = (1.486/n)(R^{2/3})(S^{1/2})$$

Then:

$$Q = (V)(A)$$

Where:

Q = capacity in cubic feet per second

V = mean velocity of flow in feet per second

A = cross sectional area in square feet

R = hydraulic radius in feet

S = slope of the energy grade line in feet per foot

n = Manning's "n" or roughness coefficient

The hydraulic radius, R, is defined as the cross sectional area of flow divided by the wetted flow surface or wetted perimeter. Allowable "n" values and maximum permissible velocities for storm sewer materials are listed in **Table 4-1**.

C. Backwater Method for Pipe System Analysis

For hydraulic analysis of existing or proposed storm drains which possess submerged outfalls, a more sophisticated design/analysis methodology than Manning's equation will be required. The backwater analysis method provides a more accurate estimate of pipe flow by calculating individual head losses in pipe systems that are surcharged and/or have submerged outlets. These head losses are added to a known downstream water surface elevation to give a design water surface elevation for a given flow at the desired upstream location. Total head losses may be determined as follows:

Total head loss = frictional loss + manhole loss + velocity head loss + junction loss

Various computer modeling programs such as HYDRA, ILLUDRAIN, and STORMCAD are available for analysis of storm drains under these conditions. Computer models to be utilized, other than those listed, must be accepted by City of Lafayette.

TABLE 4-1

Typical Values of Manning's "n"

<i>Material</i>	<i>Manning's "n"</i>	<i>Maximum Velocities (feet/second)</i>
Closed Conduits		
Concrete	0.013	10
Vitrified Clay	0.013	10
HDPE	0.012	10
PVC	0.011	10
Circular CMP, Annular Corrugations, 2 2/3 x 1/2 inch		
Unpaved	0.024	7
25% Paved	0.021	7
50% Paved	0.018	7
100% Paved	0.013	7
Concrete Culverts	0.013	10
HDPE or PVC	0.012	10
Open Channels		
Concrete, Trowel Finish	0.013	10
Concrete, Broom Finish	0.015	10
Gunite	0.018	10
Riprap Placed	0.030	10
Riprap Dumped	0.035	10
Gabion	0.028	10
New Earth ¹	0.025	4
Existing Earth ²	0.030	4
Dense Growth of Weeds	0.040	4
Dense Weeds and Brush	0.040	4
Swale with Grass	0.035	4

Source of manning "n" values: LTAP Stormwater Drainage Manual, July 1995.

¹ New earth (uniform, sodded, clay soil)

² Existing earth (fairly uniform, with some weeds)

D. Minimum Size for Storm Sewers

The minimum diameter of all storm sewers shall be 12 inches. When the minimum 12-inch diameter pipe will not limit the rate of release to the required amount, the rate of release for detention storage shall be controlled by an orifice plate or other device, subject to approval of the City of Lafayette.

E. Pipe Cover and Grade

Sewer grade shall be such that, in general, a minimum of 1.5 feet of cover is maintained over the top of the pipe. If the pipe is to be placed under pavement, then the minimum pipe cover shall be 2.0 feet from top of pavement to top of pipe. Uniform slopes shall be maintained between inlets, manholes and inlets to manholes. Final grade shall be set with full consideration of the capacity required, sedimentation problems, and other design parameters. Minimum and maximum allowable slopes shall be those capable of producing velocities of between 2.5 and 10 feet per second, respectively, when the sewer is flowing full. Maximum permissible velocities for various storm sewer materials are listed in **Table 4-1**.

F. Alignment

Storm sewers shall be straight between manholes and/or inlets.

G. Manholes/Inlets

Manholes and/or inlets shall be installed to provide human access to continuous underground storm sewers for the purpose of inspection and maintenance. The manhole/inlet access shall be no less than the jurisdictional entity standards. Manholes shall be provided at the following locations:

1. Where two or more storm sewers converge.
2. Where pipe size or the pipe material changes.
3. Where a change in horizontal alignment occurs.
4. Where a change in pipe slope occurs.
5. At intervals in straight sections of sewer, not to exceed the maximum allowed. The maximum distance between storm sewer manholes shall be as shown in **Table 4-2**.

TABLE 4-2
Maximum Distance Between Manholes

Size of Pipe (Inches)	Maximum Distance (Feet)
12 through 42	400
48 and larger	600

In addition to the above requirements, a minimum drop of 0.1 foot through manholes and inlet structures should be provided. When changing pipe size,

match crowns of pipes, unless detailed modeling of hydraulic grade line shows that another arrangement would be as effective. Pipe slope should not be so steep that inlets surcharge (i.e. hydraulic grade line should remain below rim elevation).

- Manhole/inlet inside sizing shall be as shown in **Table 4-3**.

TABLE 4-3: Manhole/Inlet Inside Sizing

Depth of Structure	Minimum Diameter	Minimum Square Opening
3-5 feet	36 inches	36" x 36"
5 or more feet	48 inches	48" x 48"

H. Inlet Sizing and Spacing

Inlets or drainage structures shall be utilized to collect surface water through grated openings and convey it to storm sewers, channels, or culverts. The inlet grate opening provided shall be adequate to pass the design 10-year flow with 50% of the sag inlet areas clogged. Inlets shall be provided so that surface water is not carried across or around any intersection. An overload channel from sag inlets to the overflow channel or basin shall be provided at sag inlets. Inlet design and spacing may be done using the hydraulic equations by manufacturers or orifice/weir equations. Use of the U.S. Army Corps of Engineers HEC-12 computer program is also an acceptable method. Gutter spread on continuous grades may be determined using the Manning's equation, or by using **Figure 4-1**. Further guidance regarding gutter spread calculation may be found in the latest edition of HERPICC Stormwater Drainage Manual, available from the Local Technical Assistance Program (LTAP). At the time of printing of this document, contact information for LTAP was:

Indiana LTAP
 Purdue University
 Toll-Free: (800) 428-7369 (Indiana only)
 Phone: (765) 494-2164
 Fax: (765) 496-1176
 Email: inltap@ecn.purdue.edu
 Website: www.purdue.edu/INLTAP/

I. Installation and Workmanship

Bedding and backfill materials around storm sewer pipes, sub-drains, and the associated structures will be installed per City of Lafayette standards. Dips/sags on newly installed storm systems will not be allowed. Variations from these standards must be justified and receive written approval from the City of Lafayette.

J. Materials

Storm sewer manholes and inlets shall be constructed of cast in place concrete or precast reinforced concrete. Material and construction shall conform to the latest edition of the Indiana Department of Transportation (INDOT) "Standard Specifications", Sections 702 and 720.

Pipe and fittings used in storm sewer construction shall be extra-strength clay pipe (ASTM C-12), ductile iron pipe (AWWA C-151), poly vinyl chloride pipe (AASHTO M252), polyethylene pipe (AASHTO M252 or AASHTO M294), or concrete pipe (AASHTO M170). Other pipe and fittings not specified herein or in Sections 907-908 of the latest edition of the INDOT "Standard Specifications" may be used only when specifically authorized by the City of Lafayette. Pipe joints shall be flexible and watertight and shall conform to the requirements of Section 906, of the latest edition of the INDOT "Standard Specifications".

K. Special Hydraulic Structures

Special hydraulic structures required to control the flow of water in storm runoff drainage systems include junction chambers, drop manholes, stilling basins, and other special structures. The use of these structures shall be limited to those locations justified by prudent planning and by careful and thorough hydraulic engineering analysis. Certification of special structures by a certified structural engineer may also be required.

L. Connections to Storm Sewer System

To allow any connections to the storm sewer system, provisions for the connections shall be shown in the drainage calculations for the system. Specific language shall be provided in the protective covenants, on the record plat, or with the parcel deed of record, noting the ability or inability of the system to accommodate any permitted connections, for example, sump pumps and footing drains.

1. **Sump pumps** installed to receive and discharge groundwater or other stormwater shall be connected to the storm sewer where possible or discharged into a designated storm drainage channel/swale. Sump pumps installed to receive and discharge floor drain flow or other sanitary sewage shall be connected to the sanitary sewers. A sump pump shall be used for one function only, either the discharge of stormwater or the discharge of sanitary sewage.
2. **Footing drains and perimeter drains** shall be connected to manholes or curb inlets, where possible, or to designated storm sewers or discharged into designated storm drainage channels/swales.

3. All **roof downspouts**, roof drains, or roof drainage piping shall discharge onto the ground or be connected to the storm drainage system. No downspouts or roof drains shall be connected to the sanitary sewers.
4. **Garage and basement floor drains** shall not be connected to the storm sewers.
5. **Swimming pool drains** shall not be connected to the storm sewers unless the water is dechlorinated, as defined in Appendix A, prior to being connected to the storm sewer.

In addition, none of the above mentioned devices shall be connected to any street underdrains, unless specifically authorized by the City of Lafayette.

M. Drainage System Overflow Design

Overflow paths throughout the development resulting from a 100-year storm event, shall be determined, clearly shown on the plans, and contained in permanent drainage easements with a minimum width of 10 feet along the centerline of the flow path. A continuous flood route from the sag inlets to the final outfall shall be shown and the minimum 10 feet along the centerline contained within an easement or road right-of-way. No fences or landscaping can be constructed within the easement areas.

The minimum adjacent grade of the portion of any residential, commercial, or industrial building (the ground elevation next to the building after construction is completed that sits adjacent to the overflow path or may be subject to flooding by the overflow path) shall be a minimum of 1 foot above the estimated 100-year elevation of the overflow path. **Table 4-4** provides minimum adjacent grades for buildings adjacent to overflow paths and minimum requirements for easement widths along overflow paths as a function of drainage area tributary to the overflow path. This table is based on 1 foot of freeboard above the water surface elevation in the overflow path and the expected width of the overflow path, using a grassed triangular channel section with 3:1 side slopes, and the difference in runoff between a 10-year frequency and 100-year frequency storm for a runoff coefficient of 0.45 and a time of concentration of 20 minutes.

In lieu of **Table 4-4**, the designer can calculate the anticipated depth of flow in the overflow system and the overflow path easement widths independently. The designer shall then submit to the City of Lafayette the overflow path depth calculations with any other necessary information to verify that the minimum adjacent grades provide approximately 1 foot of freeboard above the anticipated overflow water surface elevation and that the proposed easement widths are sufficient. If the overflow path easement widths are calculated independently, the minimum easement width shall be 10 feet. In the case of existing upstream detention, an allowance equivalent to the reduction in flow rate provided may be made for upstream detention only when: (1) such detention and release rate have previously been approved by the City of Lafayette

official charged with the approval authority at the time of the approval, and (2) evidence of its construction and maintenance can be shown.

The overflow path may be modeled as a successive series of natural ponds and open channel segments. Ponds should be modeled similar to that discussed for modeling depressional areas in Chapter 6. Channels should be modeled according to modeling techniques discussed in Chapter 5. The 100-year overflow path elevations may be based on hand calculation methods that utilize normal depth calculations and storage routing techniques or determined using computer models. Examples of computer models that either individually or in combination with other models can handle the required computations include TR-20, HEC-HMS, and HEC-1, which may be combined with HEC-RAS. Other models may be acceptable but should be approved by the City of Lafayette prior to their utilization.

If the designer utilizes **Table 4-4**, the City of Lafayette reserves the right to require independent calculations to verify that the minimum adjacent grades provide adequate freeboard above the anticipated overflow water surface elevations and to verify that sufficient overflow path easement widths are being provided. The emergency routing easement width downstream of a detention pond should be determined independently based on the proposed grading along the flow path and the discharge used in the sizing of the detention pond emergency spillway.

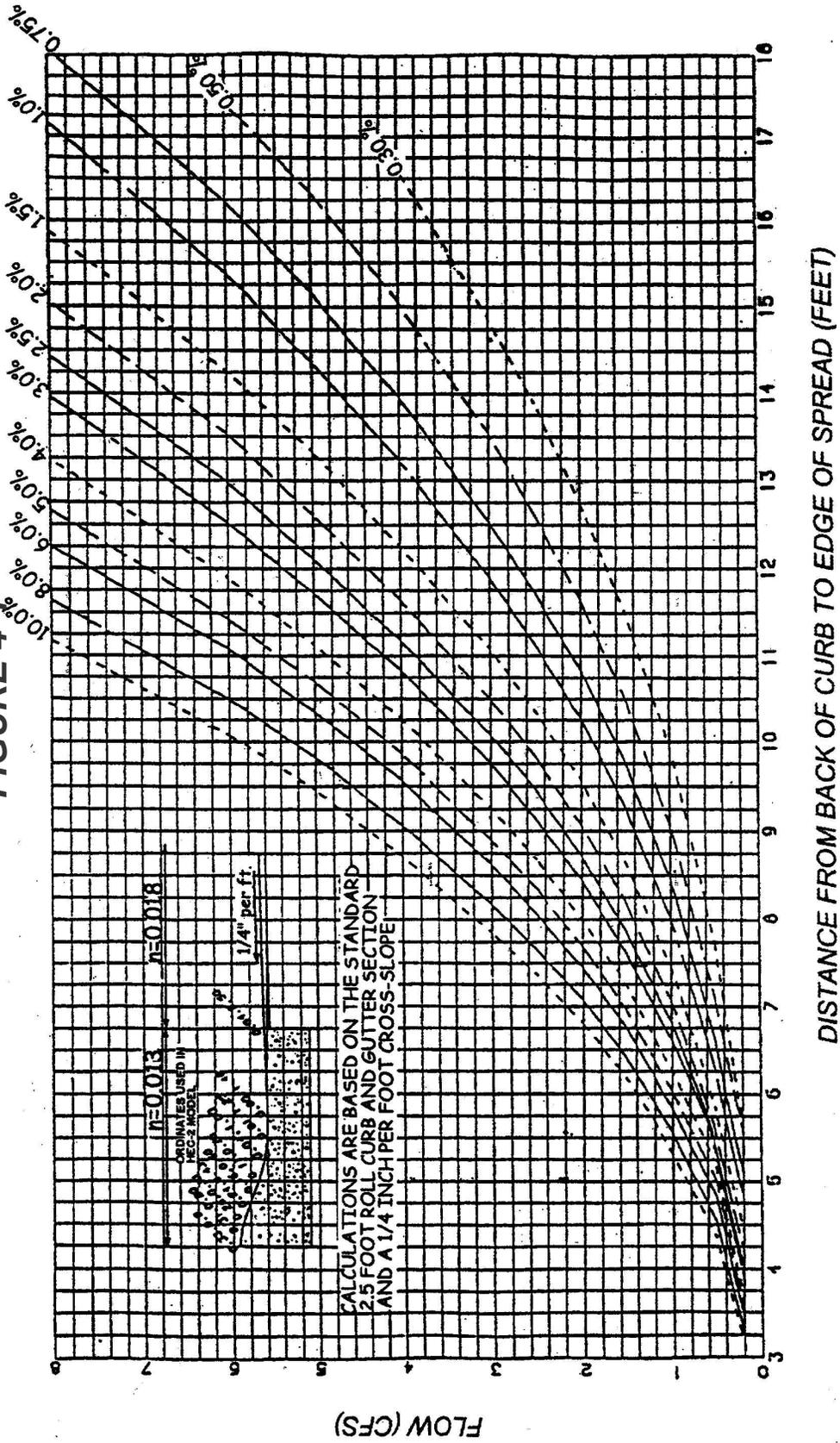
TABLE 4-4
Minimum Adjacent Grades and Easement Widths Along Overflow Paths as a Function of Drainage Area Tributary to the Overflow Path

Drainage Area (acres)	Minimum Building Adjacent Grade Above Overflow Path Invert (ft.)	Minimum Building Adjacent Grade Above Overflow Path Invert, if Overflow Path is in the Street (ft.)	Minimum Emergency Overflow Path Easement Width (ft.)
Up to 3	2.0	1.25	15
3-8	2.25	1.25	15
9-25	2.5	1.5	20
26-55	3.0	1.5	25
56-100	3.5	1.75	30
Over 100	To be calculated independently	To be calculated independently	To be calculated independently

The LAG requirements for buildings adjacent to other flooding sources are discussed elsewhere in the Ordinance or in this Manual. In case there are more than one flooding sources applicable to a building site, the highest calculated LAG for the building shall govern the placement of the building on that site.

STREET AND GUTTER CAPACITIES

FIGURE 4-1





Chapter Five

OPEN CHANNEL DESIGN STANDARDS AND SPECIFICATIONS

All channels, whether private or public, and whether constructed on private or public land, shall conform to the design standards and other design requirements contained herein. Proposed open channels must be sized and designed to convey at least the 10-year frequency on-site stormwater runoff, as well as the anticipated 10-year frequency tributary off-site stormwater runoff based on the future developed condition (using Table 6-1 of the Technical Standards). An analysis of the emergency routing of stormwater runoff through the subject development must be provided to confirm that the development will not obstruct the free flow of floodwaters from the tributary off-site property in its current condition and after development. In addition, the Drainage System Overflow Design must be completed in accordance with Section M of Chapter 4 to ensure the safe routing of flood waters through the subject development with the tributary off-site property in its current condition and after development.

A. Design Storm Frequencies

1. All channels and swales shall accommodate, as a minimum, peak runoff from a 10-year return frequency storm. For Rational Method analysis, the storm duration shall be equal to the time of concentration for the drainage area. In computer-based analysis, the duration is as noted in the applicable methodology associated with the computer program.
2. Channels with a carrying capacity of more than 30 cfs at bank-full stage shall be capable of accommodating peak runoff for a 24-hour, 50-year return frequency storm within the drainage easement.
3. Channel facilities functioning as a major drainage system, as defined in **Appendix A**, must also meet IDNR design standards in addition to the City of Lafayette standards. In case of discrepancy, the most restrictive requirements shall apply.
4. Residential rear and side lot swales shall not convey more than 4 cfs during the 10-year event. Unless designed as a post-construction stormwater quality BMP, the maximum length of rear and side lot swales before reaching any inlet shall not exceed 400 feet.

5. Regardless of minimum design frequencies stated above, the performance of all parts of drainage system shall be checked for the 100-year flow conditions to insure that all buildings are properly located outside the 100-year flood boundary and that flow paths are confined to designated areas with sufficient easement.

B. Manning's Equation

The waterway area for channels shall be determined using Manning's Equation,

$$A = Q/V$$

- where:
- A = Waterway area of channel in square feet
 - Q = Discharge in cubic feet per second (cfs)
 - V = Steady-State channel velocity, as defined by Manning's Equation (See Chapter 4)

C. Backwater Method for Drainage System Analysis

The determination of 100-year water surface elevation along channels and swales shall be based on accepted methodology and computer programs designed for this purpose. Computer programs HEC-RAS, HEC-2, and ICPR are preferred programs for conducting such backwater analysis. The use of other computer models must be approved in advance by the City of Lafayette.

D. Channel Cross-Section and Grade

1. The required channel cross-section and grade are determined by the design capacity, the material in which the channel is to be constructed, and the requirements for maintenance. A minimum depth may be required to provide adequate outlets for subsurface drains, tributary ditches, or streams. The channel grade shall be such that the velocity in the channel is high enough to prevent siltation but low enough to prevent erosion. Velocities less than 2 feet per second are not acceptable, as siltation will take place and ultimately reduce the channel cross-section area. The maximum permissible velocities in vegetated-lined channels are shown in **Table 5-1**. In addition to existing runoff, the channel design should incorporate increased runoff due to the proposed development.
2. Where depth of design flow is slightly below critical depth, channels shall have freeboard adequate to cope with the effect of hydraulic jumps.

3. Along the streets and roads, the bottom of the ditch should be low enough to install adequately-sized driveway culverts without creating "speed bumps". The driveway culvert inverts shall be designed to adequately consider upstream and downstream culvert elevations.
4. Flow of a channel into a closed system is prohibited, unless runoff rate and head loss computations demonstrate the closed conduit to be capable of carrying the 100-year channel flow for developed conditions, either entirely or in combination with a defined overflow channel, with no reduction of velocity.

TABLE 5-1: Maximum Permissible Velocities in Vegetation-Lined Channels ¹

Cover	Channel Slope Range (Percent) ³	Permissible Velocity ²	
		Erosion Resistant Soils (ft. per sec.) ⁴	Easily Eroded Soils (ft. per sec.) ⁴
Bermuda Grass	0-5	8	6
	5-10	7	5
	Over 10	6	4
Bahia Buffalo Grass Kentucky Bluegrass Smooth Brome Blue Grama	0-5	7	5
	5-10	6	4
	Over 10	5	3
Grass Mixture Reed Canary Grass	0-5 ³	5	4
	5-10	4	3
Lespedeza Sericea Weeping Lovegrass Yellow Bluestem Redtop Alfalfa Red Fescue	0-5 ⁴	3.4	2.5
	5-10		
Common Lespedeza ⁵ Sudangrass ⁵	0-5 ⁶	3.5	2.5

¹ From Soil Conservation Service, SCS-TP-61, "Handbook of Channel Design for Soil and Water Conservation".

² Use velocities exceeding 5 feet per second only where good channel ground covers and proper maintenance can be obtained.

³ Do not use on slopes steeper than 10 percent except for vegetated side slopes in combination with a stone, concrete, or highly resistant vegetative center section.

⁴ Do not use on slopes steeper than 5 percent except for vegetated side slopes in combination with a stone, concrete, or highly resistant vegetative center section.

⁵ Annuals - use on mild slopes or as temporary protection until permanent covers are established.

⁶ Use on slopes steeper than 5 percent is not recommended.

E. Side Slopes

1. Earthen channel and swale side slopes shall be no steeper than 3 horizontal to 1 vertical (3:1). Flatter slopes may be required to prevent erosion and for ease of maintenance.
2. Where channels will be lined with riprap, concrete, or other acceptable lining method, side slopes shall be no steeper than 2 horizontal to 1 vertical (2:1) with adequate provisions made for weep holes.
3. Side slopes steeper than 2 horizontal to 1 vertical (2:1) may be used for lined channels provided that the side lining is designed and constructed as a structural retaining wall with provisions for live and dead load surcharge.
4. When the design discharge produces a depth of greater than three (3) feet in the channel, appropriate safety precautions shall be added to the design criteria based on reasonably anticipated safety needs.

F. Channel Stability

1. Characteristics of a stable channel are:
 - a. It neither promotes sedimentation nor degrades the channel bottom and sides.
 - b. The channel banks do not erode to the extent that the channel cross-section is changed appreciably.
 - c. Excessive sediment bars do not develop.
 - d. Excessive erosion does not occur around culverts, bridges, outfalls or elsewhere.
 - e. Gullies do not form or enlarge due to the entry of uncontrolled flow to the channel.
2. Channel stability shall be determined for an aged condition and the velocity shall be based on the design flow or the bankfull flow, whichever is greater, using an "n" value for various channel linings as shown in **Tables 4-1 and 5-1**. In no case is it necessary to check channel stability for discharges greater than that from a 100-year frequency storm.

3. Channel stability shall be checked for conditions immediately after construction. For this stability analysis, the velocity shall be calculated for the expected flow from a 10-year frequency storm on the watershed, or the bankfull flow, whichever is smaller, and the "n" value for the newly constructed channels in fine-grained soils and sands may be determined in accordance with the "National Engineering Handbook 5, Supplement B, Soil Conservation Service" and shall not exceed 0.025. This reference may be obtained by contacting the National Technical Information Service in Springfield, Virginia. The allowable velocity in the newly constructed channel may be increased by a maximum of 20 percent to reflect the effects of vegetation to be established under the following conditions:
 - a. The soil and site in which the channel is to be constructed are suitable for rapid establishment and support of erosion controlling vegetation.
 - b. Species of erosion controlling vegetation adapted to the area, and proven methods of establishment are shown.
 - c. The channel design includes detailed plans for establishment of vegetation on the channel side slopes.

G. Drainage of Swales

Minimum swale slopes are 0.5%. All flow shall be confined to the specific easements associated with each rear and side lot swale that are part of the minor drainage system. Unless designed to act as a stormwater BMP, vegetated swales with a slope between 0.5% to 0.75% shall include paved gutters, and vegetated swales with a slope between 0.75% to less than 1.0% shall be drained with a tile system. Further guidance regarding this subject may be found in the latest edition of the Indiana Drainage Handbook.

H. Appurtenant Structures

The design of channels will include provisions for operation and maintenance and the proper functioning of all channels, laterals, travelways and structures associated with the project. Recessed inlets and structures needed for entry of surface and subsurface flow into channels without significant erosion or degradation shall be included in the design of channel improvements. The design will also provide for necessary floodgates, water level control devices, and any other appurtenance structure affecting the functioning of the channels and the attainment of the purpose for which they are built.

The effects of channel improvements on existing culverts, bridges, buried cables, pipelines, and inlet structures for surface and subsurface drainage on the channel being improved and laterals thereto shall be evaluated to determine the need for modification or replacement. Culverts and bridges which are modified or added as part of channel improvement projects shall meet reasonable standards for the type of structure, and

shall have a minimum capacity equal to the design discharge or governmental agency design requirements, whichever is greater.

I. Deposition of Spoil

Spoil material resulting from clearing, grubbing, and channel excavation shall be disposed of in a manner that will:

1. Minimize overbank wash.
2. Provide for the free flow of water between the channel and floodplain boundary unless the valley routing and water surface profiles are based on continuous dikes being installed.
3. Not hinder the development of travelways for maintenance.
4. Leave the right-of-way in the best condition feasible, consistent with the project purposes, for productive use by the owner.
5. Be approved by the IDNR or COE, if applicable.

J. Materials

Materials acceptable for use as channel lining are:

1. Grass
2. Revetment Riprap
3. Concrete
4. Hand Laid Riprap
5. Precast Cement Concrete Riprap
6. Gabions
7. Straw or Coconut Mattings (only until grass is established)

Other lining materials must be approved in writing by the City of Lafayette. Materials shall comply with the latest edition of the INDOT, "Standard Specifications".

K. Drainage System Overflow Design

See Chapter 4, Section M.



Chapter Six

STORMWATER DETENTION DESIGN STANDARDS FOR PEAK FLOW CONTROL

Basins shall be constructed to temporarily detain the stormwater runoff that exceeds the maximum peak release rate authorized by this Ordinance. The required volume of storage provided in these basins, together with such storage as may be authorized in other on-site facilities, shall be sufficient to control excess runoff from the 10-year or 100-year storm as explained below in Section “B. Also, basins shall be constructed to provide adequate capacity to allow for sediment accumulation resulting from development and to permit the pond to function for reasonable periods between cleanings.

In addition to the requirement for peak flow control through detention, the Stormwater Management Ordinance and Technical Standards require the developer to address Channel Protection and Water Quality Control requirements discussed in Chapter 8. The proper way to accommodate the water quality, channel protection, and peak flow rate control of a site is to first consider addressing the water quality and channel protection volume requirements through conventional or LID approaches (as described in Chapter 8) and then add in the required detention storage for peak flow rate control to the overall site design. Meeting the channel protection/water quality volume requirements, either using Conventional or LID Approaches, often includes providing extended detention storage features that are usually combined with the detention storage needed for peak runoff rate control of the site into one facility.

Such a combined facility should accommodate the correct channel protection volume, water quality volume, peak detention such that the allowable release rates are met, and the appropriate drain times. The following is the suggested calculation sequence for designing a detention pond with combined extended detention and peak flow control detention functions.

[Note: Developments with disturbed areas that are 10 acres or less and/or discharge directly into a stream without providing detention do not require Channel Protection Volume (CPv) measures.]

1. Calculate the required extended detention storage volume as needed to address the on-site Channel Protection Volume (CPv) through the methodology provided in Chapters 8 and 9 (Depending on the approach utilized, there may be no need for providing an extended detention storage).

2. Determine the control elevation/invert for the drain serving the proposed extended detention storage (ensuring a positive drain to the site outlet). This will be the elevation of the bottom of the proposed extended detention storage and top of the permanent pool if a wet bottom pond is being provided for.
3. Design a storage space to accommodate the extended detention storage volume determined in Step 1, assuming 0.0 cfs going through the drain that will serve this extended detention storage volume. The top of this storage space will be the bottom of the peak flow rate control detention storage and the invert of the main outlet of this peak flow rate control detention storage.
4. Design the main outlet of the peak flow rate control detention storage, sized to carry the allowable on-site 10-year and 100-year release rates, with its control elevation/invert at the top of the extended detention storage space. For storage space calculation purposes, use the actual orifice size calculated to accommodate the release rates regardless of whether the orifice size is smaller or larger than the minimum orifice size allowed under the Ordinance or Technical Standards.
5. Design the peak flow rate control detention storage space by routing the on-site 10-year and 100-year inflow hydrographs through the pond, assuming the pond is empty to the control elevation of the extended detention storage drain as determined in Step 2, but still assuming 0.0 cfs can get out of the extended detention storage drain as the pond fills up. The resulting maximum water surface elevation is where the invert of the emergency overflow weir (sized for 1.25 times the peak inflow rate) is located. The pond size and control elevation/invert elevations are final at this stage.
6. Determine the size of the extended detention storage drain and design the drain system in a manner to meet the extended detention minimum and maximum emptying time requirements discussed in Chapter 8, using both on-site and, if applicable, off-site runoff. If the calculated orifice size of the drain system outlet is less than the minimum orifice size allowed in the Ordinance or these Technical Standards, designate the minimum orifice size on the construction plans, but do not go back and recalculate/redesign the storage spaces.
7. To make sure that the addition of the release through the drain will not cause the allowable release rate to be exceeded, reroute the on-site and off-site 10-year and 100-year inflow hydrographs through the pond, this time allowing water to also leave through the extended detention storage drain as the pond fills up. If the total peak outflow discharge exceeds the allowable release rate, reduce the size of the main outlet orifice accordingly (but do not go back to redesign the storage space). The resulting maximum water surface elevation is the 100-year pool elevation.

8. If the calculated orifice size of the main peak flow rate control storage outlet is less than the minimum orifice size allowed in the Ordinance or these Technical Standards, designate the minimum orifice size on the construction plans, but do not go back and recalculate/redesign the storage spaces.

The following shall govern the design of any improvement with respect to the detention of stormwater runoff for peak flow control.

A. Acceptable Detention Facilities

The increased stormwater runoff resulting from a proposed development should be detained on-site by the provisions of appropriate above- or below-ground wet bottom or dry bottom detention facilities, parking lots, or other acceptable techniques. Measures that retard the rate of overland flow and the velocity in runoff channels shall also be used to partially control runoff rates.

B. Allowable Release Rates

1. General Release Rates

Control devices shall limit the discharges to a rate such that the post-developed release rates from the site are no greater than the general release rates provided in Table 6-1. The general release rates may be set at a lower value by the City of Lafayette for certain watersheds if more detailed data becomes available as a result of comprehensive watershed studies conducted and/or formally approved and adopted by the City of Lafayette. The applicant shall confirm the applicable release rates with the City of Lafayette prior to initiating the design calculations to determine whether a basin-specific rate has been established for the watershed.

For sites where the pre-developed area has more than one (1) outlet, the release rates should be computed based on the pre-developed discharge to each outlet point.

2. Site-Specific Release Rates for Sites with Depressional Storage

For sites where depressional storage exists, the general release rates provided above may have to be further reduced. If depressional storage exists at the site, site-specific release rates must be calculated according to methodology described in Chapter 2, accounting for the depressional storage by modeling it as a pond whose outlet is a weir at an elevation that stormwater can currently overflow the depressional storage area. Post-developed release rates for sites with depressional storage shall be the 2-year pre-developed peak runoff rate for the post-developed 10-year storm and the 10-year pre-developed peak runoff rate for the post-developed 100-year storm. In no case shall the calculated site-specific release rates be larger than the general rates provided above.

Note that by definition, depressional storage does not have a direct gravity outlet, but if in agricultural production, it is more than likely drained by a tile and should be modeled as “empty” at the beginning of a storm. The function of any existing depressional storage should be modeled using an event hydrograph model to determine the volume of storage that exists and its effect on the existing site release rate. To prepare such a model, certain information must be obtained, including delineating the tributary drainage area, the stage-storage relationship and discharge-rating curve, and identifying the capacity and elevation of the outlet(s).

The tributary area should be delineated on the best available topographic data. After determining the tributary area, a hydrologic analysis of the watershed should be performed, including, but not limited to: a calculation of the appropriate composite runoff curve number and time of concentration. Stage-storage data for the depressional area should be obtained from the site topography. The outlet should be clearly marked and any calculations performed to create a stage-discharge rating curve must be included with the stormwater submittal.

Also, note that for determining the post-developed peak runoff rates, the depressional storage must be assumed to be filled unless the City of Lafayette can be assured that the noted storage will be preserved in perpetuity.

3. Management of Off-site Runoff

Runoff from all upstream tributary areas (off-site land areas) may be bypassed around the detention/retention facility without attenuation. Such runoff may also be routed through the detention/retention facilities provided that a separate outlet system or channel is incorporated for the safe passage of such flows, i.e., not through the primary outlet of a detention facility. Unless the pond is being designed as a regional detention facility and therefore all off-site runoff to the pond detained, the primary outlet structure shall be sized and the invert elevation of the emergency overflow weir determined according to the on-site runoff only. To accomplish this, the 100-year on-site runoff must be determined by temporarily ignoring the off-site runoff and routing it through the pond and through the primary outlet pipe. The resulting pond elevation would be the invert elevation of the emergency overflow weir. Once the size and location of the primary outlet structure as well as the invert elevation of the emergency overflow weir is determined by considering on-site runoff, the 100-year pond elevation is determined by routing the entire inflow, on-site and off-site, through the pond. Note that the total peak flow released from the outlet system shall not be larger than the total of the allowable release rate and the off-site flow being bypassed through the pond for the 100-year event. A separate emergency overflow spillway would then need to be placed at the 100-year pond elevation and the top of the dam elevation selected with a minimum freeboard of 1 foot. It should be noted that the efficiency of the detention/retention facility in controlling the on-site runoff may be severely affected if the off-site area is considerably larger than the on-site area. As a general guidance, on-line detention may not be effective in

controlling on-site runoff where the ratio of off-site area to on-site area is larger than 5:1. Additional detention (above and beyond that required for on-site area) may be required by the City of Lafayette when the ratio of off-site area to on-site area is larger than 5:1.

4. Downstream Restrictions

In the event the downstream receiving channel or storm sewer system is inadequate to accommodate the post-developed release rate provided above, then the allowable release rate shall be reduced to that rate permitted by the capacity of the receiving downstream channel or storm sewer system. Additional detention, as determined by the City of Lafayette, shall be required to store that portion of the runoff exceeding the capacity of the receiving sewers or waterways.

If the proposed development makes up only a portion of the undeveloped watershed upstream of the limiting restriction, the allowable release rate for the development shall be in direct proportion to the ratio of its drainage area to the drainage area of the entire watershed upstream of the restriction.

In accordance with Chapter 6, Section 8 of the Stormwater Management Ordinance, the allowable release rates may be further reduced by the City of Lafayette if on-site or off-site conditions warrant the reduction.

As an alternative to reduction of release rates, the City of Lafayette may allow the applicant to pursue alleviating downstream restrictions. The applicant would be responsible for obtaining all permits and consents required and for incurring all expenses involved in such undertaking.

**TABLE 6-1
General Release Rates (cfs/acre) Based on
Pre-Developed Condition Curve Number at Each Outlet Point**

Curve Number	10-Year	100-Year
≤ 65	0.07	0.23
66 – 69	0.13	0.33
70 – 73	0.20	0.43
74 – 77	0.27	0.53
78 – 81	0.34	0.63
82 – 85	0.44	0.75
≥ 86	0.55	0.87

5. Documentation of Results

The results of the allowable release rate determinations as well as the modeling simulation results must be summarized in a table that shall be included in the

Stormwater Drainage Technical Report. The table must include, for each eventual site outlet, the pre-developed acreage tributary to each eventual site outlet, the unit discharge allowable release rate used, the resulting allowable release rate in cfs for the post-developed 10-year and 100-year events, as well as pre- and post-developed flow rates for the 2-,10- and 100-year events. The worksheet provided as Table 6-2 should be filled for each final site outlet.

TABLE 6-2

SITE OUTLET #	ITEM	PRE-DEVELOPMENT					POST-DEVELOPMENT				
		D.A. (ac)	Depress. Storage? (yes/no)	2- Yr.	10- Yr.	100- Yr.	D.A. (ac)	Depress. Storage? (yes/no)	2- Yr.	10- Yr.	100- Yr.
1	General Allowable Release Rate (cfs/acre)										
	Basin-Specific Unit Discharge Allowable Release Rate, if any (cfs/acre)										
	Unit Discharge Allowable Release Rate Based on D/S Restrictions, if any (cfs/acre)										
	Adopted Unit Discharge Allowable Release Rate (cfs/acre)										
	Contributing Area of Development Site (ac) and Allowable Release Rate (cfs)										
	Total Contributing DA (ac) and Modeling Results (cfs)										

C. General Detention Basin Design Requirements

1. The detention facility shall be designed in such a manner that a minimum of 90% of the maximum volume of water stored and subsequently released at the design release rate shall not result in a storage duration in excess of 48 hours from the start of the storm unless additional storms occur within the period. In other words, the design shall ensure that a minimum 90% of the original detention capacity is restored within 48 hours from the start of the design 100-year storm. When conditions exist such that the detention basin emptying requirement and the allowable release rate from the project site cannot both be met, the applicant shall develop a detention basin design that is as close as possible to meeting both requirements, with the design fully supported in the stormwater drainage technical report. In most cases, when conditions exist as described above, priority should be given to matching the allowable release rate.
2. The 100-year elevation of stormwater detention facilities shall be separated by not less than 25 feet from any building or structure to be occupied. The Lowest Adjacent Grade (including walkout basement floor elevation) for all residential, commercial, or industrial buildings shall be set a minimum of 2 feet above the 100-year pond elevation, or 2 feet above the emergency overflow weir elevation, whichever is higher. The decision by the owner to include a basement for any residential, commercial, or industrial building shall be dictated by site soil conditions, the water table, and other related on-site considerations.
3. No detention facility or other water storage area, permanent or temporary, shall be constructed under or within twenty (20) feet of any pole or high voltage electric line. Likewise, poles or high voltage electric lines shall not be placed within twenty (20) feet of any detention facility or other water storage area.
4. All stormwater detention facilities shall be separated from parking lots and roadways by a designed method of safety barrier, such as guard rails, bollards, or other physical barriers, or existing buffers, capable of deterring passage of a vehicle into the pond.
5. Slopes no steeper than 3 horizontal to 1 vertical (3:1) for safety, erosion control, stability, and ease of maintenance shall be permitted.
6. Safety screens having a maximum opening of four (4) inches shall be provided for any pipe or opening 18" in diameter, or larger, to prevent children or large animals from crawling into the structures.
7. Use of fences around all detention ponds is strongly encouraged to assure safety.

Unless specifically required by the City of Lafayette, the decision to use fencing around detention ponds is left to the owner or the developer. Recommendations contained within this document do not relieve the applicant

and owner/developer from the responsibility of taking all necessary steps to ensure public safety with regards to such facilities.

8. Outlet control structures shall be designed to operate as simply as possible and shall require little or no maintenance and/or attention for proper operation. For maintenance purposes, pond outlets shall be a minimum of 0.5 foot above the normal water level of the receiving water bodies. They shall limit discharges into existing or planned downstream channels or conduits so as not to exceed the predetermined maximum authorized peak flow rate. For above ground facilities, if an outlet control structure includes an orifice to restrict the flow rate, such orifice shall be no less than 4 inches in diameter, even if the 4-inch diameter orifice results in a discharge that exceeds the predetermined maximum authorized peak flow release rates. However, the sizing of the pond still should be based on the more restrictive maximum allowable release rate. Storm sewer pipes that act as outlet control structures from detention ponds shall be reinforced concrete pipe.
9. Emergency overflow facilities such as a weir or spillway shall be provided for the release of exceptional storm runoff or in emergency conditions should the normal discharge devices become totally or partially inoperative. The overflow facility shall be of such design that its operation is automatic and does not require manual attention.

Emergency overflow facilities shall be designed to convey, without overtopping the detention facility banks, one and one-quarter (1.25) times the peak inflow discharge resulting from the 100-year design storm event runoff from the entire contributing watershed draining to the detention facility, assuming post-development condition on-site and existing condition off-site. The length of the weir is to be determined using the weir equation, with the overflow weir control elevation at the pond's 100-year elevation (pond is assumed full to the overflow weir control elevation), discharge equal to 1.25 times the peak 100-year inflow, and the maximum head being the difference between the weir control elevation and the top of the bank.

The emergency overflow routing from the emergency overflow facility to an adequate receiving system must be positive (by gravity) and shown on the construction plans and on the plat. It must be sized to accommodate the design flow of the pond's emergency overflow weir. Thirty (30) feet along the centerline of this emergency overflow route shall be designated as permanent drainage easement. No fences or landscaping can be constructed within the easement areas. The Lowest Adjacent Grade of all residential, commercial, or industrial buildings along this emergency overflow route shall be set a minimum of 2 feet above the flood elevation along the route, calculated based on the pond's emergency overflow weir design discharge.

10. Grass or other suitable vegetative cover shall be provided along the banks of the detention storage basin. Vegetative cover around detention facilities should be maintained as appropriate.
11. Debris and trash removal and other necessary maintenance shall be performed on a regular basis to assure continued operation in conformance to design.
12. For wet-bottom facilities; A dedicated permanent stormwater easement extending 20 feet from the 100-year frequency ponding limits will surround the facility. The drainage easement shall be accessible from a public right-of-way and shall be dedicated and granted to the persons or entities authorized to maintain the facility. In no case shall the easement be dedicated to the City of Lafayette. The individual lot owner shall be responsible for maintaining that part of the facility easement which exists on his or her property. No trees, playground equipment, fencing, mini-barns, swimming pools, landscaping appurtenances, or other potential obstructions may be placed within the facility easement. Written evidence in the form of covenants that run with and become a charge against the land providing for the perpetual maintenance of the stormwater easement shall be submitted to the City of Lafayette, which said covenant shall be recorded with the County Recorder. The permanent water level, facility outlet, and facility emergency spillway will be placed within a common area under the responsibility of a lot owner's association for the subdivision in which the facility is located.

For dry-bottom facilities; The 100-year frequency ponding limits, facility outlet, and facility emergency spillway will be placed within a common area under the responsibility of a lot owner's association for the subdivision in which the facility is located.

D. Additional Requirements for Wet-Bottom Facility Design

Where part of a detention facility will contain a permanent pool of water, all the items required for detention storage shall apply. Also, a controlled positive outlet will be required to maintain the design water level in the wet bottom facility and provide required detention storage above the design water level. However, the following additional conditions shall apply (For visual clarification, refer to **Figures 6-1** and **6-2**):

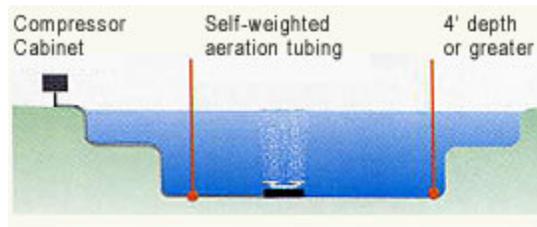
1. Facilities designed with permanent pools or containing permanent lakes shall have a water area of at least one-half (0.5) acre with a minimum depth of eight (8) feet over the majority of pond area. If fish are to be used to keep the pond clean, a minimum depth of approximately ten (10) feet shall be maintained over at least 25 percent of the pond area. Construction trash or debris shall not be placed within the permanent pool.

2. Prior to final acceptance, danger signs warning of deep water, possible flood conditions during storm periods, or other dangers per the City of Lafayette standards, shall be installed at an interval of 400 feet around the perimeter of wet-bottom facilities without a security fence. Rescue equipment stations shall be posted at the same locations as the warning signs. Maintenance of the warning signs and rescue equipment stations shall be the responsibility of the owner or the homeowners association.
3. For wet-bottom facilities with a security fence, a maintenance ledge six (6) feet in width is required and shall be installed approximately 12 inches above the permanent water level. A safety ledge six (6) feet in width is also required, and shall be installed approximately 18 inches below the permanent water level. The slope between the two ledges shall be stable and protected from erosion with hard armoring or bioengineered techniques.

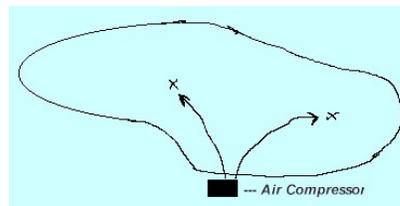
For wet-bottom facilities without a security fence, a safety ledge ten (10) feet in width is required and shall be installed on a slope no steeper than 5 horizontal to 1 vertical (5:1), with the outer edge of the safety ledge no deeper than 18 inches below the permanent water level. The ground surface shall be no steeper than 3:1 from the top of slope to the safety ledge, and from the safety ledge to the bottom of the facility.

4. A safety ramp exit from the lake shall be required in all cases and shall have a minimum width of twenty (20) feet and exit slope of 6 horizontal to 1 vertical (6:1). The ramp shall be constructed of suitable material to prevent structural instability due to vehicle impacts and/or wave action.
5. The planting of vegetated barriers, from the top of bank to the waters edge around the perimeter of wet-bottom detention facilities without a security fence, is highly recommended. These barriers serve as potential obstacles to persons who may consider entering the water. The seed mixes used to plant the barrier should be able to survive in long dry periods as well as potential long wet periods. The vegetation should be planted in a manner so as not to hide or disguise the ponds edge. Maintenance of the vegetated barrier shall be the responsibility of the owner or the homeowners association.
6. Parks, playgrounds, and athletic fields shall be separated from all stormwater detention facilities by no less than 100 feet, measured from the 100-year pool of the pond. Trails and sidewalks shall be separated from all stormwater detention facilities by no less than 25 feet, measured from the 100-year pool of the pond.
7. Periodic maintenance is required in lakes to control weed and larval growth. The facility shall also be designed to provide for the easy removal of sediment that will accumulate during periods of reservoir operation. A means of maintaining the designed water level of the lake during prolonged periods of dry weather may also be required.

8. Methods to prevent pond stagnation, including but not limited to surface or sub-surface aeration or destratification facilities that can, at the minimum, achieve one complete pond volume turnover per day should be considered. Figure below shows a typical diffuser aeration system that consists of a quiet air compressor at the shore, aeration tubing, and one or more sets of diffuser head(s).



Irregularly shaped ponds should be treated as two or three separate ponds. Likewise, larger ponds will also need multiple aeration units.



Design calculations to substantiate the effectiveness of proposed aeration facilities shall be submitted with final engineering plans. Design calculations should, at a minimum, indicate that the device and or series of devices are providing a minimum volume turnover of once per 24 hours over the majority of the pond volume (i.e. pump radius of influence calculations, etc.). Agreements for the perpetual operation and maintenance of aeration facilities shall be prepared to the satisfaction of the City of Lafayette.

If the detention pond is also proposed to be used as a post-construction stormwater BMP, alternative means of aeration (such as diffuser aeration systems) shall be used that would not result in re-suspension of sediment particles and would not prevent the efficient settling of sediment particles

E. Additional Requirements for Dry-Bottom Facility Design

In addition to general design requirements, detention facilities that will not contain a permanent pool of water shall comply with the following requirements:

1. Provisions shall be incorporated into facilities for complete interior drainage of dry bottom facilities, including the provisions of natural grades to outlet structures,

longitudinal and transverse grades to perimeter drainage facility, paved gutters, or the installation of subsurface drains.

2. For residential developments, the maximum planned depth of stormwater stored shall not exceed four (4) feet.
3. In excavated detention facilities, a minimum side slope of 3:1 shall be provided for stability. In the case of valley storage, natural slopes may be considered to be stable.

F. Parking Lot Storage

Paved parking lots may be designed to provide temporary detention storage of stormwater on all or a portion of their surfaces. Outlets for parking lot storage of stormwater will be designed so as to empty the stored waters slowly. Depths of storage shall be limited to a maximum depth of seven (7) inches so as to prevent damage to parked vehicles and so that access to parked vehicles is not impaired. Ponding should in general, be confined to those positions of the parking lots farthest from the area served. Before such detention method is allowed, a perpetual maintenance agreement must be executed by the owner or the developer and filed with the City of Lafayette. In addition, the 100-year inundation boundary should be determined and clearly shown on the construction plans.

G. Detention Facilities in Floodplains

Placement of detention ponds within the 100-year floodplain is not appropriate. In rare cases when the City of Lafayette may allow detention storage within a 100-year floodplain, only the net increase in storage volume above that which naturally existed on the floodplain shall be credited to the development. Wet-bottom detention storage facilities located within the 100-year floodplain may eliminate the floodplain storage that existed on the detention pond site. Therefore, compensatory floodplain storage may need to be provided elsewhere for the floodplain storage lost as a result of the wet-bottom detention pond. A detention storage facility located within the 100-year floodplain and utilizing a backflow prevention device will eliminate the floodplain storage that existed on the detention pond site, and will also require compensatory floodplain storage. The detention analysis for a detention pond in the floodplain must consider appropriate tailwater impacts and the effect of any backflow prevention device.

H. Joint Development of Control Systems

Stormwater control systems may be planned and constructed jointly by two or more developers as long as compliance with this Ordinance is maintained.

I. Diffused Outlets

When the allowable runoff is released in an area that is susceptible to flooding or erosion, the developer may be required to construct appropriate storm drains through such area to avert increased flood hazard caused by the concentration of allowable runoff at one point instead of the natural overland distribution. The requirement of diffused outlet drains shall be at the discretion of the City of Lafayette.

J. IDNR Requirements

All designs for basins to be constructed in the floodway of a stream with a drainage area of one square mile or more must also satisfy IDNR permit requirements.

K. Allowance for Sedimentation

Detention basins shall be designed with an additional six (6) percent of available capacity to allow for sediment accumulation resulting from development and to permit the pond to function for reasonable periods between cleanings. Basins should be designed to collect sediment and debris in specific locations so that removal costs are kept to a minimum. For wet-bottom ponds, the sediment allowance may be provided below the permanent pool elevation. No construction trash or debris shall be allowed to be placed within the permanent pool. If the pond is used as a sediment control measure during active construction, the performance sureties will not be released until sediment has been cleaned out of the pond and elevations and grades have been reestablished as noted in the approved plans.

**FIGURE 6-1
Wet –Bottom Detention Facility – With Fence**

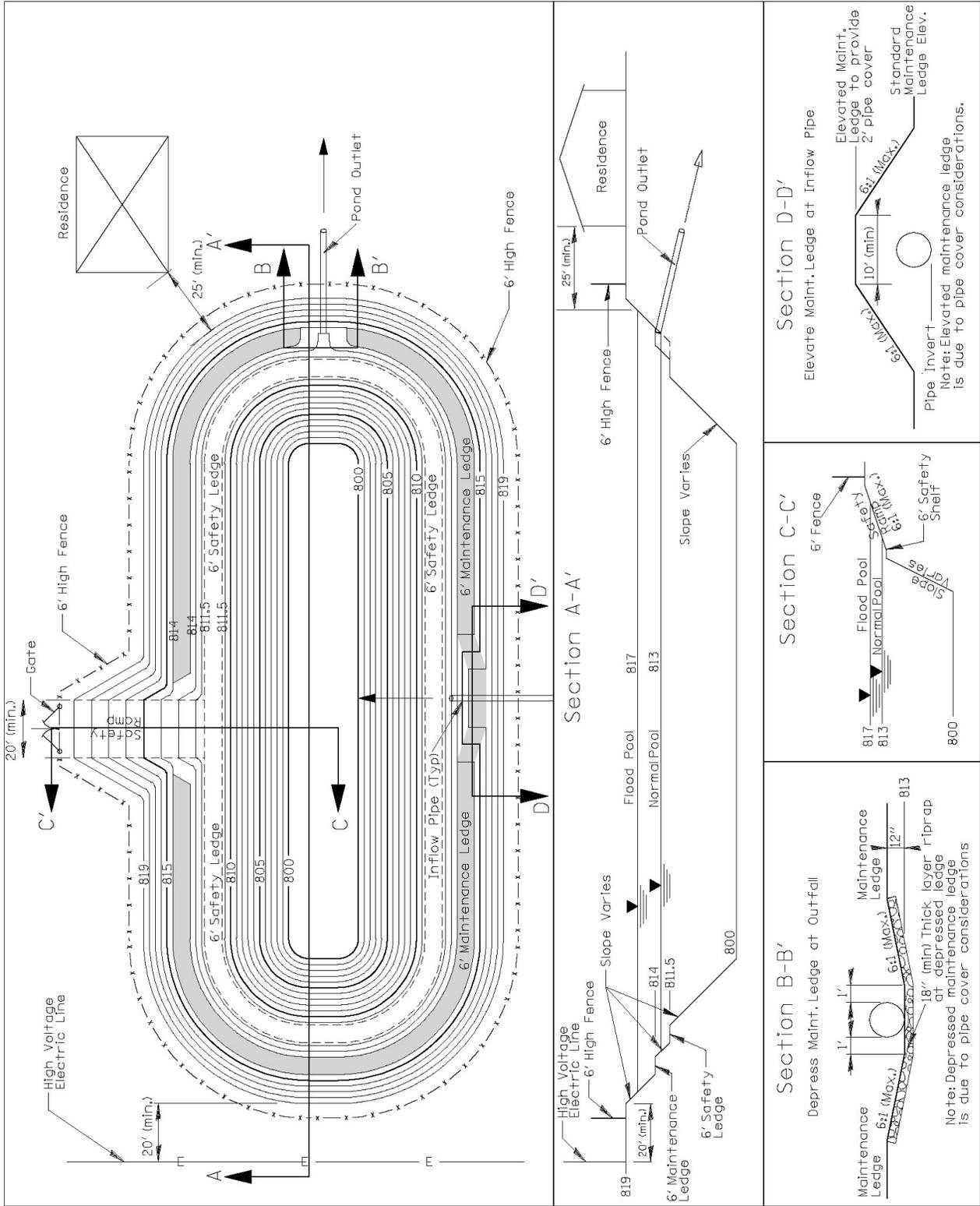
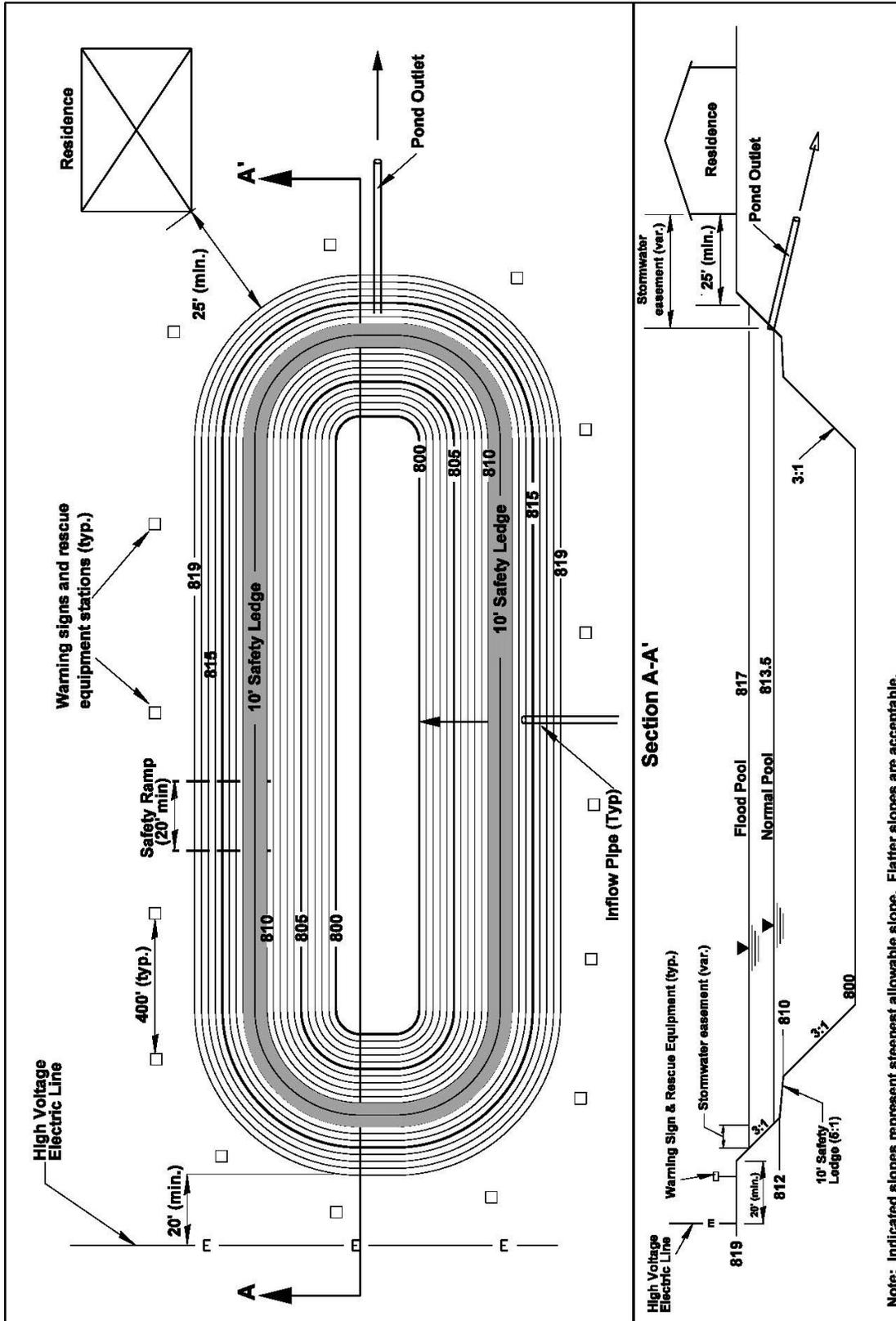


FIGURE 6-2
Wet -Bottom Detention Facility – Without Fence



Note: Indicated slopes represent steepest allowable slope. Flatter slopes are acceptable.



Chapter Seven

EROSION CONTROL PRACTICES AND CONSTRUCTION PHASE BMPs

The requirements contained in this chapter are intended to prevent stormwater pollution resulting from soil erosion and sedimentation or from mishandling of solid and hazardous waste. Practices and measures included herein should assure that no foreign substance, (e.g. sediment, construction debris, chemicals) be transported from a site and allowed to enter any drainageway, whether intentionally or accidentally, by machinery, wind, rain, runoff, or other means.

A. POLLUTANTS OF CONCERN DURING CONSTRUCTION

The major pollutant of concern during construction is sediment. Natural erosion processes are accelerated at a project site by the construction process for a number of reasons, including the loss of surface vegetation and compaction damage to the soil structure itself, resulting in reduced infiltration and increased surface runoff. Clearing and grading operations also expose subsoils which are often poorly suited to re-establish vegetation, leading to longer term erosion problems.

Problems associated with construction site erosion include: transport of pollutants attached to transported sediment; increased turbidity (reduced light) in receiving waters; recreational use impairment. The deposited sediment may pose direct toxicity to wildlife, or smother existing spawning areas and habitat. This siltation also reduces the capacity of waterways, resulting in increased flood hazards to the public.

Other pollutants of concern during the construction process are hazardous wastes or hydrocarbons associated with the construction equipment or processes. Examples include concrete washoff, paints, solvents, and hydrocarbons from refueling operations. Poor control and handling of toxic construction materials pose an acute (short-term) or chronic (long-term) risk of death to both aquatic life, wildlife, and the general public.

B. EROSION AND SEDIMENT CONTROL REQUIREMENTS

General and Implementation Requirements - The following general and implementation requirements apply to all land-disturbing activities and shall be considered in the preparation of a SWPPP within the corporate boundaries of the City of Lafayette.

1. Trained Individuals must be utilized for activities associated with the development and design of the SWPPP, stormwater measure implementation, and stormwater project management.
2. Minimize the potential for soil erosion by designing a development that fits the topography and soils of the site. Unless needed to meet requirements and goals of the development, steep slopes should be avoided, and natural contours should be followed.
3. All activities on a site should be conducted in a logical sequence and in accordance with the site's construction phasing plan so that the smallest practical area of land will be exposed for the shortest practical period of time during development.
4. The length and steepness of designed slopes should be minimized to reduce erosion potential. Drainage channels and swales must be designed and adequately protected so that their final gradients and resultant velocities will not cause erosion in the receiving channel or at the outlet. Methods for determining acceptable velocities are included in this Stormwater Technical Standards Manual as well as in the IDEM Storm Water Quality Manual.
5. Sediment-laden water which otherwise would flow from the project site shall be managed by appropriate erosion and sediment control measures to minimize sedimentation to receiving waters and adjacent properties as discussed in the IDEM Storm Water Quality Manual and other authoritative sources.
6. Public roadways and roadways not exclusive to construction traffic shall be kept cleared of accumulated sediment that is a result of runoff or tracking. The following minimum conditions are applicable:
 - a. Clearing of sediment must not include the utilization of mechanical methods that will result in mobilization of dust off the project site or flushing the area with water unless the flushed water is directed to an appropriate sediment control measure.
 - b. Cleared sediment must be redistributed or disposed of in a manner that is in accordance with all applicable statutes and regulations.
 - c. Sediment discharged or tracked onto roadways that are open to traffic must be removed as directed by a regulatory authority or at a minimum, removed by the end of the same day.
7. Phasing of construction activities must be used, when feasible, to minimize the footprint of disturbed unstable areas.
8. Collected runoff leaving a project site must be either discharged directly into a well-defined, stable receiving channel, or diffused and released to adjacent

property without causing an erosion or pollutant problem to the adjacent property owner.

9. Natural features, including wetlands and sinkholes (karst features), shall be protected from pollutants associated with stormwater runoff, through appropriate stormwater management and/or treatment measures.
10. Soil compaction is to be minimized, especially in areas where permanent vegetation will be re-established and/or areas that are designated to infiltrate stormwater for the post-construction phase.
11. Topsoil must be preserved, unless infeasible.
12. Existing natural buffers that are adjacent to waters of the state must be preserved to promote infiltration and provide protection of the water resource, unless infeasible. Activities performed by a county drainage board under IC 36-9-27 are excluded.
 - a. Natural buffers must be preserved, including the entire buffer bordering and/or surrounding the water resource. Existing buffers:
 - i. 50 feet or more in width must be preserved to a minimum of 50 feet.
 - ii. less than 50 feet in width must be preserved in their entirety. May be enhanced with vegetation that is native and promotes ecological improvement and sustainability.
 - b. Runoff directed to the natural buffer must be:
 - i. treated with appropriate erosion and sediment control measures prior to discharging to the buffer.
 - ii. managed with appropriate runoff control measures to prevent erosion from occurring within the buffer area.
 - c. Further information regarding buffer requirements is contained in IDEM's "Implementation of Buffers" guidance document.
13. Minimize the generation of dust through dust suppression techniques to prevent deposition into waters of the state and areas located beyond the permitted boundaries of the site as discussed in the IDEM Storm Water Quality Manual and other authoritative sources.
14. A stable construction site access measure must be provided at all points of construction traffic ingress and egress to the project site. Where the selected measure is not effective, an alternative measure or additional controls must be

utilized to minimize tracking. Alternative measures may include, but are not limited to, wheel wash systems and rumble strips.

15. During the period of construction activities, all stormwater management measures necessary to meet the requirements of this permit must be maintained. Alternative measures must be selected and implemented, as necessary.
16. Discharge water from dewatering of ground water from excavations, trenches, foundations, etc. must not be discharged when:
 - a. Sediment-laden water is not first directed to an appropriate sediment control measure or a series of control measures, as per IDEM Storm Water Quality Manual and other authoritative sources, that minimizes the discharge of the sediment.
 - b. A visible sheen and/or pollutants are present at a level that requires additional treatment and/or an alternate permit.
17. Appropriate measures must be implemented to eliminate wastes or unused building materials including, but not limited to garbage, debris, cleaning wastes, wastewater, concrete washout, mortar/masonry products, soil stabilizers, lime stabilization materials, and other substances from being carried from a project site by runoff or wind. Wastes and unused building materials must be managed and disposed of in accordance with all applicable statutes and regulations.
18. Construction and domestic waste must be managed to prevent the discharge of pollutants and windblown debris. Surplus plastic or hardened concrete/cementitious materials are not required to be placed in trash receptacles and are considered clean fill that may be reused, disposed of on-site, or recycled in accordance with applicable state and federal regulations. Management of waste materials may include, but are not limited to:
 - a. Waste containers (trash receptacles), when selected to manage waste, must be managed to reduce the discharge of pollutants and blowing of debris. Receptacles that are not appropriately managed will require alternatives that include but are not limited to:
 - i. A cover (e.g., lid, tarp, plastic sheeting, temporary roof) to minimize exposure of wastes to precipitation or
 - ii. A similarly effective method designed to minimize the discharge of pollutants.
 - b. Waste that is not disposed of in trash receptacles must be protected from exposure to the weather and/or removed at the end of the day from the site and disposed of properly.

19. Concrete and cementitious wash water areas, where cementitious fluids are permissible, must be identified for the site and the locations clearly posted. Wash water must be directed into leak-proof containers or leak-proof containment areas which are located and designed to divert runoff away from the measure and sized to prevent the discharge and/or overflow of the cementitious wash water. If not evaporated, wash water must be removed (pumped) for appropriate off-site disposal.
20. Fertilizer applications associated with the stabilization plan for the project must meet the following requirements:
 - a. Apply fertilizer at a rate and amount as determined by a soil analysis or in accordance with the Indiana Stormwater Quality Manual or similar guidance documents.
 - b. Apply fertilizer at an appropriate time of year for the project location, taking into consideration proximity to a waterbody, and preferably timed to coincide with the period of maximum vegetative uptake and growth.
 - c. Avoid applying fertilizer immediately prior to precipitation events that are anticipated to result in stormwater runoff from the application area.
21. Proper storage and handling of materials, such as fuels or hazardous wastes, and spill prevention and clean-up measures must be implemented to minimize the potential for pollutants to contaminate surface or ground water or degrade soil quality. To meet this requirement:
 - a. A spill prevention and response plan, meeting the requirements in 327 IAC 2-6.1, must be completed.
 - b. Proper project management and the utilization of appropriate measures including, but not limited to, eliminating a source or the exposure of materials must be completed.
 - c. Manage the following activities:
 - i. Fueling and maintenance of equipment.
 - ii. Washing of equipment and vehicles.
 - iii. Storage, handling, and disposal of construction materials, products, and wastes.
 - iv. Application of pesticides, herbicides, insecticides, and fertilizers
 - v. Dispensing and utilization of diesel fuel, oil, hydraulic fluids, other petroleum products, and other chemicals.
 - vi. Handling and disposal of hazardous wastes, including, but not limited to paints, solvents, petroleum-based products, wood preservatives, additives, curing compounds, and acids.
 - vii. Washing of applicators and containers used for paint, grout, or other materials.
22. Personnel associated with the project must be informed of the terms and conditions of this permit and the requirements within the SWPPP. The permittee

is required to document this process. Information must be provided through written notification, contracts, or other means (i.e., pre-construction meetings) that effectively communicates the provisions and requirements of the permit and SWPPP. Personnel may include, but are not limited to:

- a. General contractors, construction management firms, grading or excavating contractors, and trade industry representatives (i.e. concrete industry) associated with the overall project.
 - b. Contractors or individual lot operators that have primary oversight on individual building lots.
 - c. Those responsible for the implementation of the SWPPP, and the installation, repair, and maintenance of stormwater measures.
 - d. Those responsible for the application and storage of treatment chemicals.
 - e. Those responsible for administering the self-monitoring program (SMP).
23. A notice must be posted near the main entrance of the project site or at a publicly accessible location. For linear project sites, such as a pipeline or highway, the notice must be placed in a publicly accessible location near the project field office.

The notice must be maintained in a legible condition and include:

- a. A copy of the completed IDEM NOI or a document, such as the Permit Summary Report & Notice of Sufficiency letter produced by IDEM's online ePortal system, that at a minimum contains the name and address of the owner, name and location address of the construction project, and where would the runoff from project directed to (stream(s) receiving the discharge(s).
 - b. The NPDES permit number(s), upon receipt.
 - c. The location of the construction plan/SWPPP if the project site does not have an on-site location to store the plan.
24. The use of anionic polymers (cationic polymers are not authorized for use) on the project site are authorized for sediment control provided their use is in conformance with current State of Indiana standards and specifications and the use is identified in the stormwater pollution prevention plan (SWPPP). If use of a polymer is not in the SWPPP and is selected at a later date, notification to IDEM and the City of Lafayette is required. An email notification prior to the use of the polymer to the IDEM Stormwater Program is acceptable. For projects regulated by a MS4 notification must follow the local process for the use of polymers.
25. Restoration and/or clean-up may be required for those areas impacted by sediment or other pollutant discharges. These activities will be performed as directed by the inspecting authority and may require:

- a. Development and submittal of a removal and restoration plan to ensure the methodology chosen will not result in further degradation of the resource.
- b. Permission by a property owner when the restoration activity requires access to a property owned by another entity or individual.
- c. Additional permits prior to initiation of the work.

Stabilization Requirements - The following stabilization requirements apply to all land-disturbing activities:

1. Un-vegetated areas that are left idle or scheduled to be left inactive must be temporarily or permanently stabilized with measures appropriate for the season to minimize erosion potential. To meet this requirement, the following apply:
 - a. Stabilization must be initiated by the end of the seventh day the area is left idle. The stabilization activity must be completed within fourteen (14) days after initiation. Initiation of stabilization includes, but is not limited to, the seeding and/or planting of the exposed area and applying mulch or other temporary surface stabilization methods where appropriate. Areas that are not accessible due to an unexpected and disruptive event that prevents construction activities are not considered idle.
 - b. Areas that have been compacted may be excluded from the stabilization requirement when the areas are intended to be impervious surfaces associated with the final land use, provided runoff from the area is directed to appropriate sediment control measures.
2. Final stabilization of a project site is achieved when:
 - a. All land-disturbing activities have been completed and a uniform (evenly distributed, without large bare areas) perennial vegetative cover with a density of seventy percent (70%) has been established on all unpaved disturbed areas, and areas not covered by permanent structures, or equivalent permanent stabilization measures have been employed. This requirement does not apply to:
 - i. Landscaping that is part of the final project plan is considered stable when the plan has been fully implemented and areas not being vegetated are stable with a non-erosive material and/or product.
 - ii. Projects or specific stormwater measures that utilize native vegetation and/or special vegetative plantings that are either required by a water quality permit/authorization or part of the design

and functionality of a stormwater measure provided the activity does not pose a threat that will result in off-site sedimentation.

- iii. Projects on land used for agricultural purposes when:
 - a) Stabilization is completed in accordance with the above Stabilization Requirements (in 1. a. and b.) as land-disturbance progresses. Land that is returned to agricultural production must be temporarily or permanently seeded upon completing land-disturbing activities. Stabilization requirements may be waived by the inspecting authority if the project site does not pose a threat of discharging sediment.
 - b) Disturbed areas, not previously used for agricultural production, such as filter strips, must be returned to their pre land disturbance use.
- b. Specific projects, due to function and/or operation may necessitate that an area remain disturbed. Only the minimum operational area is allowed to remain disturbed. This option primarily applies to off-road recreational commercial operations but may apply to other land use types upon determination by the regulating entity.

Design Requirements - The following design requirements apply to all land-disturbing activities and shall be considered in the selection, design, and implementation of all stormwater quality and management measures contained in the SWPPP:

1. Sound engineering, agronomic, and scientific principles must be utilized for measures contained in the SWPPP.
2. Appropriate measures must be planned, designed, and installed as part of an erosion and sediment control system and in accordance with the site's construction phasing plan.
3. Stormwater runoff leaving the project site must be discharged in a manner that is consistent with this ordinance, state, or federal law.
4. Collected runoff leaving the project site must be directed to an established vegetated area, when feasible and applicable, to increase pollutant removal and maximize stormwater infiltration and then either discharged directly into a well-defined, stable receiving conveyance or diffused and released without causing erosion at the point of discharge.
5. Conveyance systems must be designed taking into consideration both peak flow and total volume and must be adequately protected so that their final gradients and resultant velocities are unlikely to cause erosion at the outlet or in the

receiving channel, based on known conditions of the discharge at the time of design to accommodate post-construction conditions.

6. Sediment basins, where feasible, must withdraw water from the surface of the water column unless equivalent sediment reduction can be achieved by use of alternative measures. Alternative measures include but are not limited to increasing the basin length to width ratio to 4:1 or greater, implementation of porous baffles, use of flocculants/polymers, and/or phasing of project land disturbance that also incorporates a rapid stabilization program. During freezing conditions, the implementation of alternative withdrawal methods may be utilized.

Monitoring and Management Requirements - A trained individual, acceptable to the City of Lafayette, shall monitor project construction and stormwater activities. These shall include:

1. A written evaluation of the entire project site, with the exception of those areas that are considered unsafe. The evaluation must be performed by a trained individual and completed:
 - a. Twenty-four (24) hours prior to a qualifying precipitation event or by the end of the next business day following each measurable storm event (excludes accumulated snow events); which is defined as a precipitation accumulation equal to, or greater than, one-half (0.50) inch of rainfall within a 24-hour period. If no rain event occurs within the work week a minimum of one inspection must occur. In the event of multiple qualifying events during the work week, no more than three (3) inspections would be required to meet the self-monitoring commitment.
 - b. At a minimum of one (1) time per month for specific areas within the project which are stabilized with permanent vegetative cover at seventy (70) percent density and/or erosion resistant armoring is installed. A reduction to once per month is also applicable for the entire project site for stabilized common areas, basins, conveyances, outfalls, and inactive building sites. Prior to reducing the monitoring to monthly, records must identify the area and the date the area became eligible for monthly monitoring. Weekly monitoring as identified in (a) above must resume if one or more of the following occurs:
 - i. The vegetative cover fails or there is evidence of erosion in the identified area.
 - ii. The City of Lafayette requires monitoring to resume.
2. A complete written evaluation report which must include:
 - a. Name of the individual performing the evaluation, including printed name, title, and signature (electronic signatures are acceptable).

- b. Date of the evaluation.
- c. Amount of precipitation, when the evaluation is conducted after a measurable storm event. Recorded rainfall may be documented utilizing an on-site rain gauge or storm event information from a weather station that is representative of the project location.
- d. Observations of project performance in relation to:
 - i. Implementation of the stormwater pollution prevention plan.
 - ii. Assessment of existing stormwater measures based on industry standards and maintenance standards as identified in Section 5 of the Stormwater Permit Application Form (found in Appendix B1 of this document) to ensure each measure is operational and functioning properly.
 - iii. Additional measures necessary in the event an existing measure fails or is not present in the landscape
 - iv. Impacts including, but not limited to, sediment discharges, erosion, discharges that results in bank erosion, and operational activities that have the potential to generate pollutants and unauthorized discharges.
- e. Documentation of an actual discharge that is visible during the assessment, the location of the discharge and a visual description of the discharge. The visual description includes, but is not limited to, color (turbidity reading is an option), odor, floatables, settled/suspended solids, foam, oil sheen, and any other visible sign that may be attributed to operations occurring on the project site.
- f. Detail of corrective action recommended and/or completed. Corrective action includes, but is not limited to:
 - i. Repairing, modifying, or replacing any stormwater management measure.
 - ii. Clean-up and proper disposal of spills, releases, or other deposits.
 - iii. Remedying a permit violation.
 - iv. Taking reasonable steps to remediate, minimize or prevent the discharge of pollutants associated with the construction activity until a permanent corrective solution is initiated.
 - v. Restoring an impacted area and/or removing accumulated sediment, provided appropriate permission and permits are obtained to conduct the activity.
- g. A timeline for which the corrective action will occur to remediate the discharge of pollutants. The established corrective action, at a minimum, must be initiated:

- i. On the day the deficiency was discovered or when it is not practical to initiate on the discovery date, no later than forty- eight (48) hours for the repair of a measure or installation of a temporary measure until a new and/or replacement measure is installed as specified in item ii) below.
 - ii. Within seven (7) days of discovery for the installation of a new (alternative) measure or replacement of an existing measure unless a shorter timeframe is required as part of a regulatory inspection. The inspecting authority may also allow additional time to take corrective action.
 - iii. If corrective action cannot be achieved within the timelines outlined in i) or ii) above, a reason for incompleteness must be provided and documented, including the anticipated completion date.
 - h. Documentation of corrective action taken from the previous self-monitoring report.
3. Maintaining the SMP reports at the site or at an easily accessible location (refer to Project Documentation Requirements below).
 4. Providing all reports for the project site to the City of Lafayette within forty-eight (48) hours of a request. Electronic copies are acceptable, provided they are in a format consistent with the paper record.

Project Documentation Requirements – The following project documentation shall be developed and maintained:

1. Maintain a project management log that contains:
 - a. Information related to all off-site borrow sites, disposal areas, and staging areas, including the location of each activity as it is identified and/or selected.
 - b. Information related to all project activities including, but not limited to:
 - i. SMP reports.
 - ii. Regulatory inspections.
 - iii. Responses to a compliance action or enforcement action.
 - iv. Records showing the dates of all SWPPP modifications. The records must include the name of the person authorizing each change and a summary of all changes.
2. Ensure the SWPPP and supporting documentation associated with the SMP and project management log are accessible at the project site office or in the possession of on-site individuals with responsibility for the overall project management or associated with the management and operations of construction

activities. This information must be provided to *the* City of Lafayette within forty-eight (48) hours of a request.

C. COMMON CONTROL PRACTICES

All erosion control and stormwater pollution prevention measures required to comply with this Ordinance shall meet the design criteria, standards, and specifications similar to or the same as those outlined in the “Indiana Drainage Handbook” and the “Indiana Storm Water Quality Manual (ISWQM),” or other comparable and reputable references. Table 7-1 lists some of the more common and effective practices for preventing stormwater pollution from construction sites. Details of each practice can be found in the Indiana Drainage Handbook, the ISWQM, or in Appendix C. These practices should be used to protect **every** potential pollution pathway to stormwater conveyances.

**Table 7-1
Common Stormwater Pollution Control Practices for Construction Sites**

Practice No.	BMP Description	Applicability	Fact Sheet
1	Site Assessment	All sites	ISWQM (Ch.2)
2	Development Of A Construction Sequence Schedule	All sites	ISWQM (Ch. 5)
3	Tree Preservation and Protection	Nearly all sites	ISWQM
4	Temporary Construction Ingress/Egress Pad	All sites	ISWQM
5	Wheel Wash	All sites	CN - 101
6	Silt Fence	Small drainage areas	ISWQM
7	Surface Roughening	Sites with slopes that are to be stabilized with vegetation	ISWQM
8	Temporary Seeding	Areas of bare soil where additional work is not scheduled to be performed for a minimum of 15 days	ISWQM
9	Mulching	Temporary surface stabilization	ISWQM
10	Erosion Control Blanket (Surface)	Temporary surface stabilization, anchor for mulch	ISWQM
11	Temporary Diversion	Up-slope and down-slope sides of construction site, above disturbed slopes within site	ISWQM
12	Rock Check Dam	2 acres maximum contributing drainage area	ISWQM
13	Temporary Slope Drain	Sites with cut or fill slopes	ISWQM
14	Straw Bale Dam	Small drainage areas	ISWQM
15	Geotextile Fabric Drop Inlet Protection	1 acre maximum contributing drainage area	ISWQM
16	Insert (Basket) Curb Inlet Protection	1 acre maximum contributing drainage area	ISWQM
17	Stone Bag Curb Inlet Protection	1 acre maximum contributing drainage area	ISWQM
18	Temporary Sediment Trap	5 acre maximum contributing drainage area	ISWQM
19	Temporary Dry Sediment Basin	30 acre maximum contributing drainage area	ISWQM
20	Dewatering Structures	Sites requiring dewatering	CN-102
21	Dust Control	All sites	ISWQM
22	Spill Prevention and Control	All sites	CN - 103
23	Solid Waste Management	All sites	CN - 104
24	Hazardous Waste Management	All sites	CN - 105

* See ISWQM Chapter 7 (2007 or latest version), unless otherwise noted. (<http://www.in.gov/idem/stormwater/>)

D. INDIVIDUAL LOT CONTROLS

Although individual lots within a larger development may not appear to contribute as much sediment as the overall development, the cumulative effect of lot development is of concern. From the time construction on an individual lot begins, until the individual lot is stabilized, the builder must take steps to:

1. protect adjacent properties from sedimentation,
2. prevent mud/sediment from depositing on the street,
3. protect drainageways from erosion and sedimentation, and
4. prevent sediment laden water from entering storm sewer inlets.

This can be accomplished using numerous erosion and sediment control measures. A standard erosion control plan for individual lots is provided in Appendix B. The standard plan includes perimeter silt fence, stabilized construction entrance, curb inlet protection, drop inlet protection, stockpile containment, stabilized drainage swales, downspout extensions, temporary seeding and mulching, and permanent vegetation. Every relevant measure should be installed at each individual lot site.

Construction sequence on individual lots should be as follows:

1. Clearly delineate areas of trees, shrubs, and vegetation that are to be undisturbed. To prevent root damage, the areas delineated for tree protection should be at least the same diameter as the crown.
2. Install perimeter silt fence at construction limits. Position the fence to intercept runoff prior to entering drainage swales.
3. Avoid disturbing drainage swales if vegetation is established. If drainage swales are bare, install erosion control blankets or sod to immediately stabilize.
4. Install drop inlet protection for all inlets on the property.
5. Install curb inlet protection, on both sides of the road, for all inlets along property frontage and the along the frontage of adjacent lots.
6. Install gravel construction entrance that extends from the street to the building pad.
7. Perform primary grading operations.
8. Contain erosion from any soil stockpiles created on-site with silt fence around the base.
9. Establish temporary seeding and straw mulch on disturbed areas.
10. Construct the home and install utilities.
11. Install downspout extenders once the roof and gutters have been constructed. Extenders should outlet to a stabilized area.
12. Re-seed any areas disturbed by construction and utilities installation with temporary seed mix within 3 days of completion of disturbance.
13. Grade the site to final elevations.
14. Install permanent seeding or sod.

All erosion and sediment control measures must be properly maintained throughout construction. Temporary and permanent seeding should be watered as needed until established. For further information on individual lot erosion and sediment control,

please see the “Individual Lot Erosion and Sediment Control Certification” form in Appendix B, the latest information posted on IDEM’s website, and the City of Lafayette standard drawings.



Chapter Eight

POST-CONSTRUCTION WATER QUALITY BMPs

A. INTRODUCTION

It is recognized that developed areas, as compared to undeveloped areas, generally have increased imperviousness, decreased infiltration rates, increased runoff rates, and increased concentrations of pollutants such as fertilizers, herbicides, greases, oil, salts and other pollutants. As new development and re-development continues within the corporate boundaries of the City of Lafayette, measures must be taken to intercept and remove pollutants from stormwater runoff prior to reaching regional creeks, streams, and rivers. Through the use of appropriate Best Management Practices (BMPs), harmful amounts of sediment, nutrients, and contaminants will be removed from stormwater.

City of Lafayette has adopted a policy that the control of stormwater runoff quality will be based on the management of Total Suspended Solids (TSS). This requirement is being adopted as the basis of the City of Lafayette stormwater quality management program for all areas of jurisdiction. The following table provides the post-construction stormwater quality best management practice requirements for development sites within the City of Lafayette:

**Table 8-1
Post-Construction Stormwater Quality Best Management Practice Requirements**

Disturbed Area	Post-Construction BMP Requirement
0 to less than ½ acre	No BMP
At least ½ acre to less than 3 acres	At least 1 BMP to provide 80% TSS removal ^A
At least 3 acres	At least 1 BMP to provide 80% TSS removal OR 2 BMPs ^{B,C}

Notes:

- A. Since pollutant loadings generally increase with an increase in developed area, at least one BMP is required to provide 80% TSS removal from sites with a disturbed area of at least ½ acre to less than 3 acres.
- B. A combination of at least two BMPs is encouraged, but not required for sites with a disturbed area of at least 3 acres. The first BMP will act as a pretreatment measure to reduce TSS concentrations within the downstream, or secondary, BMP. A dual BMP provision provides a failsafe benefit should adverse conditions result in undue clogging or other potential BMP impairment. Only one of the required BMPs can be a proprietary Water Quality Device.
- C. Those sites with a disturbed area in excess of 3 acres need only provide one BMP to provide 80% TSS removal and do not need to provide channel protection volume (described in Section A, below) when any of the following apply:

- a. Stormwater runoff from the subject site is tributary to an existing regional detention facility designed by the jurisdictional entity
- b. Stormwater runoff from the subject site is tributary to existing stormwater drainage infrastructure constructed as part of the larger overall development, previously approved by the jurisdictional entity, and designed to receive stormwater runoff from the developed subject site
- c. Stormwater runoff from the subject site is tributary to an existing detention pond constructed as part of a different development previously approved by the jurisdictional entity, provided that:
 - i. Stormwater runoff from the subject site is tributary to the existing detention facility in the pre-developed condition
 - ii. The existing detention pond was designed to receive stormwater runoff from the developed subject site

It is also recognized that another major source of pollution in many Indiana streams, including those within the corporate boundaries of the City of Lafayette, is the streambank erosion associated with urbanizing watersheds. Stream channels develop their shape in response to the volume and rate of runoff that they receive from their contributing watersheds. Research has shown that in hydrologically stable watersheds, the stream flow responsible for most of the shaping of the channel (called the bankfull flow) occurs between every one to two years. When land is developed, the volume and rate of runoff from that land increases for these comparatively small flooding events that are not normally addressed by detention practices and the stream channel will adapt by changing its shape. As the stream channel works to reach a new stable shape, excess erosion occurs. As new development and re-development continues within the corporate boundaries of the City of Lafayette, measures must be taken to minimize the impact of such development or re-development on streambank erosion. Through the use of appropriate Best Management Practices (BMPs), the volume and rate of runoff for channel-forming flows will be reduced in an attempt to minimize increased streambank erosion in the receiving streams and channels.

Channel protection is typically achieved by matching the post-construction runoff volume and rate to the pre-settlement condition (the condition prior to any historical land conversion by man) for all runoff events up to the bankfull flow. The bankfull flow in most Indiana streams correlate with 1.5- to 2-year flood event flow. However, due to difficulties in determining the pre-settlement conditions, the net control of runoff resulting from a 1-year, 24-hour storm in proposed conditions (rather than the alternative method of determining increase in 2-year, 24-hour storm over pre-settlement conditions) for sites that are equal or larger than 10 acres is established as the City of Lafayette standard for channel protection.

Requirements of the Ordinance and this Technical Standards Manual with regard to channel protection and water quality protection can be satisfied through a variety of methods broadly categorized under two general approaches:

- Conventional Approach
- Low Impact Development (LID) Approach

This chapter of the manual establishes minimum standards for the selection and design of post-construction water quality and channel protection BMPs. The information provided in this chapter establishes performance criteria for stormwater quality

management and procedures to be followed when preparing a BMP plan for compliance. Post-construction BMPs must be sized to treat the channel protection volume (CPv), water quality volume (WQv), and for flow-through BMPs, the water quality discharge rate (Qwq). For the conventional approach, the methodology for calculating the CPv, WQv, and Qwq values is provided in Chapter 9. For the LID approach, alternative methods of calculating these values are provided in Section C of this chapter.

[Note: Developments discharging directly into a stream without providing detention do not require Channel Protection Volume (CPv) measures. Developments discharging directly to a regional detention facility will be evaluated on a case-by-case basis.]

B. CONVENTIONAL APPROACH PROCEDURES

The following procedures shall be followed according to the conventional approach:

Step 1: Provide BMPs to address Channel Protection Volume

In a conventional approach, the receiving channel is protected through extended detention of the 1-year, 24-hour storm event for the entire site (disturbed and undisturbed) tributary to each outlet. The methodology for calculating the Channel Protection Volume (CPv) is according to that discussed in Chapter 9 of this Standards Manual. Both wet and dry extended detention may be used so long as only 10% of the maximum stored volume is left in the basin after 36 hours from maximum storage time and no more than 40% of the maximum stored volume is released within the first 12 hours. To ensure adequate detention volume is available within the facility over time, the facility should be designed for long-term sediment accumulation (a minimum of 50 years). If long-term sediment accumulation cannot be adequately provided for in the pond, or if the pond is intended to provide sediment control during the construction phase of the project, forebays near inlets can be included to help manage sediment accumulation. Forebays do not require a hard maintenance surface and shall not be visibly disconnected from the pond by rip rap or other berm structures.

Since by design 90% of the original volume will be available within 48 hours of the start of each storm event (assumed to be about 36 hours from when the Channel Protection pool is full), the volume in the pond associated with the channel protection (CPv) may be assumed empty for the purpose of peak flow detention analysis discussed in Chapter 6. In addition, the volume provided for channel protection would also satisfy the water quality volume (WQv) requirement for one (1) post-construction water quality BMP provided that the facility meets the design criteria in the fact sheet.

Step 2: Provide BMPS to address Water Quality Management

A channel protection volume BMP can also be used as a post-construction stormwater quality BMP if it meets the design criteria in Appendix D. City of Lafayette has designated a number of pre-approved BMP methods (listed in Tables 8-2 through 8-7 and 8-9) to be combined in various ways to achieve the stormwater quality performance criteria noted in Section A of this chapter. BMP methods address the runoff generated from not more than the first inch of rainfall for the entire site (disturbed and undisturbed) tributary to each outlet. These BMP measures are listed along with their anticipated average pollutant removal rates in **Table 8-2**. Applicants desiring to use a different removal rate for these BMPs must follow the requirements for Innovative BMPs. Details regarding the applicability and design of these pre-approved BMPs, including the effectiveness of these BMPs in removing TSS, are contained within fact sheets presented in Appendix D1. Additional information on recommended plant lists and recommended materials used for construction of stormwater BMPs are provided in Appendix D2 and Appendix D3, respectively.

Innovative BMPs, including but not limited to, BMPs not previously accepted by the City of Lafayette must be certified by a Professional Engineer or Professional Land Surveyor licensed in the state of Indiana and approved through the City of Lafayette. ASTM standard methods must be followed when verifying performance of new measures. New BMPs, in all possible combinations, must meet the performance criteria noted in Section A of this chapter, including the capture and removal of floatables. All innovative BMPs must have a low to medium maintenance requirement to be considered by the City of Lafayette. Testing to establish the pollutant removal rate must be conducted by an independent testing facility, not the BMP manufacturer. The accepted design flow rate for a Water Quality Device shall be the flow value at which the claimed removal rate for each pollutant is equaled or exceeded based on the unit's efficiency curve (flow rate versus removal rate graph).

**TABLE 8-2
Pre-approved Post-Construction BMPs**

BMP ^A	Typical % TSS Removal Efficiency ^B	Maintenance Easement Requirements
Bioretention	90 ^C	25 feet wide along the perimeter
Constructed Wetland	67 ^C	25 feet wide along the outer perimeter of forebay & 30 feet wide along centerline of outlet
Underground Detention	70	20 feet wide strip from access easement to tank's access shaft & 30 feet wide along centerline of inlet and outlet
Extended Detention/Dry Pond	72	25 feet wide along the outer perimeter of forebay & 30 feet wide along centerline of outlet
Infiltration Basin	87	25 feet wide along the perimeter
Infiltration Trench	90 ^C	25 feet wide along the perimeter
Constructed (Sand) Filter	70 ^C	25 feet wide along the perimeter
Water Quality Device	NA ^D	20 feet wide strip from access easement to chamber's access shaft
Vegetated Filter Strip	78 ^C	25 feet wide along the length on the pavement side
Vegetated Swale	81 ^C	25 feet wide along the top of bank on one side
Wet Ponds/Retention Basin	80	25 feet wide along the outer perimeter of forebay & 30 feet wide along centerline of outlet

Notes:

- A. Detailed specifications for these BMPs are provided in the fact sheets contained in Appendix D1.
- B. Removal rates shown are based on typical results. Unless otherwise shown, data extracted by CBBEL from various data sources. These rates are also dependent on proper installation and maintenance. The ultimate responsibility for determining whether additional measures must be taken to meet the Ordinance requirements for site-specific conditions rests with the applicant.
- C. Based on IDEM Stormwater Quality Manual, 2007.
- D. The removal rate for this category varies widely between various models and manufacturers. Independent testing must be provided, rather than the manufacturer's testing data. In lieu of Independent testing data, the latest pre-approved proprietary BMPs list from the city of Indianapolis will be used. These BMPs must be configured as offline units. The accepted design flow rate for a Water Quality Device shall be the flow value at which the claimed removal rate for each pollutant is equaled or exceeded based on the unit's efficiency curve (flow rate versus removal rate graph).

C. LOW IMPACT DEVELOPMENT (LID) APPROACH PROCEDURES

Low impact development (LID) stormwater management design approaches are fundamentally different from conventional design approaches and challenge traditional thinking regarding development standards, watershed protection, and public participation. LID combines fundamental hydrologic concepts with many of today's common stormwater strategies, practices and techniques to reshape development patterns in a way that maintains natural watershed hydrologic functions. When the [jurisdictional entity] has a stormwater user fee system based on imperviousness, the utilization of LID concepts also often results in a smaller stormwater user fee for non-residential lots. The five principles of LID are:

1. Conservation of existing natural and topographic features;
2. Minimization of land clearing and impervious surfaces;
3. Maintain or lengthen the pre-developed time of concentration;
4. Installation of integrated structural best management practices; and
5. Use of pollution prevention measures and practices.

Several methods for achieving the above requirements and principals are outlined below. In addition to methods described in this Standards Manual, several readily available references provide details on incorporating LID practices into site development. One of the most recent, comprehensive resources for incorporating LID practices into site development design is "Low Impact Development Manual for Michigan: A Design Guide for Implementers and Reviewers" available online at www.semcoq.org/LowImpactDevelopment.aspx. The noted resource was used extensively for the development of the LID section in this Standards Manual.

The following steps shall be followed for the LID approach:

1. **Minimize Disturbed Areas and Protect Sensitive Areas**
 - Map sensitive areas such as waterbodies, floodplains, and natural flow paths. Identify existing structures and infrastructure. Identify hydrologic soil types on the maps. Show elevations and identify critical slopes of 15 percent to 25 percent and above 25 percent. Show areas of known contamination. Also show existing structures and infrastructure.
 - Determine the total area of impervious surface existing prior to development.
 - Note the seasonal high groundwater level.
 - Designate sensitive areas that are proposed to be protected as part of the proposed layout.
 - Lay out the proposed development, minimizing disturbance and avoiding the sensitive areas.
 - The non-structural BMPs listed in **Table 8-3** are designed to properly protect sensitive areas so they maintain their pre-development state and runoff characteristics. Fact sheets for these BMPs are provided in Appendix D1.

**Table 8-3
Pre-approved BMPs with Treatment Area Reduction Recognitions for LID
Approach**

BMP ^A	Runoff Reduction Recognition ^B
Protect Sensitive Areas	Area (acres complying with the requirements of this BMP) can be subtracted from site development area for Channel Protection Volume and Water Quality Volume/Rate calculations.
Protect Riparian Buffers	Area (acres complying with the requirements of this BMP) can be subtracted from site development area for Channel Protection Volume and Water Quality Volume/Rate calculations.
Minimize Total Disturbed Area	Area (acres complying with the requirements of this BMP) can be subtracted from site development area for Channel Protection Volume and Water Quality Volume/Rate calculations.
Reduce Impervious Surfaces	Area (acres complying with the requirements of this BMP) can be subtracted from site development area for Channel Protection Volume and Water Quality Volume/Rate calculations.
Protect Natural Flow Pathways	Area (acres complying with the requirements of this BMP) can be subtracted from site development area for Channel Protection Volume and Water Quality Volume/Rate calculations.
Cluster-Type Development	Area (undisturbed acres complying with the requirements of this BMP) can be subtracted from site development area for Channel Protection Volume and Water Quality Volume/Rate calculations.

Notes:

- A. In using and recognizing the runoff reduction of these BMPs, applicants must meet the review criteria located within the discussion of each BMP provided in Appendix D1.
 - B. If the LID track is pursued, reduced CNs (associated with pre-developed underlying soil types instead of the normal requirement of assigning the post-development CN according to the next lower infiltration soil group) for areas protected by these BMPs may be used for determining the post-developed runoff rates and volumes for larger events (up to and including the 100-year event). See **Table 8-9**.
- As shown in Table 8-3, when using the LID Approach, any area that is set aside and protected as described in those BMPs may be subtracted from site development area for purposes of determining Channel Protection Volume calculations and water quality volume/rate calculations.
 - In addition, for determining the 10-year and 100-year runoff and peak discharges, the CN associated with the original, pre-development soil groups (instead of the normal requirement of assigning the post-development CN according to the next lower infiltration soil group) may be used for these areas (see **Table 8-9**).

- The runoff reduction recognition only works with designs based on the Curve Number or CN method of analysis utilizing non-composite CN determination methods.

2. **Restore Disturbed Areas**

- For the LID Approach, runoff reduction recognitions are used in the design process to emphasize the use of BMPs that, when applied, restore/alter the disturbed area in a way that reduces the volume of runoff from that area.
- Runoff reduction recognition is provided for the five BMPs listed in **Table 8-4** because they enhance the response of a piece of land to a storm event rather than treat the runoff that is generated. These BMPs are encouraged because they are relatively easy to implement over structural controls, require little if any maintenance, and the land they are applied to remains open to other uses.
- Runoff reduction recognition is applied by reducing the default CN value so that the amount of runoff generated from an event is reduced.
- The runoff reduction recognition only works with designs based on the Curve Number or CN method of analysis utilizing non-composite CN determination methods.
- Fact sheets for these BMPs are provided in Appendix D1.

Table 8-4
Pre-approved BMPs with Runoff Reduction Recognitions for Restoring Disturbed
Areas as Part of LID Approach

BMP ^A	Runoff Reduction Recognition ^B
Minimize Soil Compaction	Area (acres complying with the requirements of this BMP) can be assigned a CN based on the Pre-developed soil group conditions instead of the normal requirement of assigning the post-development CN according to the next lower infiltration soil group.
Protection of Existing Trees within disturbed areas (part of Protect Sensitive Areas)	Trees protected under the requirements of this BMP can be assigned a CN based on the Pre-developed soil group conditions at a rate of 800 square feet per tree instead of the normal requirement of assigning Post-developed CN according to the next lower infiltration soil group for the acres covered by the tree area.
Soil Amendment and Restoration	Area (acres complying with the requirements of this BMP) can be assigned a CN based on the Pre-developed soil group conditions instead of the normal requirement of assigning the post-development CN according to the next lower infiltration soil group.
Native Revegetation	Proposed trees and shrubs over and above the requirements of the landscaping ordinance to be planted under the requirements of this BMP can be assigned a CN based on the Pre-developed soil group conditions at a rate of 200 square feet per tree and 25 square feet per shrub instead of the normal requirement of assigning Post-developed CN according to the next lower infiltration soil group for the acres covered by the existing land use area.
Riparian Buffer Restoration	Proposed trees and shrubs to be planted under the requirements of this BMP can be assigned a CN based on the Pre-developed soil group conditions at a rate of 200 square feet per tree and 25 square feet per shrub instead of the normal requirement of assigning Post-developed CN according to the next lower infiltration soil group for the acres covered by the existing land use area.

Notes:

- A. In using and recognizing the runoff reduction of these BMPs, applicants must meet the review criteria located within the discussion of each BMP provided in Appendix D1.
- B. If the LID track is pursued, reduced CNs (associated with pre-developed underlying soil types instead of the normal requirement of assigning the post-development CN according to the next lower infiltration soil group) for areas covered by these BMPs may be used for determining the post-developed runoff rates and volumes for larger events (up to and including the 100-year event). See **Table 8-9**.

3. **Minimize Imperviousness**

- The BMPs listed in **Table 8-5** are designed to reduce the volume of runoff from hard surfaces such as roads, sidewalks, parking areas, roofs, etc. For the LID approach, runoff reduction recognition is used to encourage these practices and recognize their runoff reduction impacts. Fact sheets for these BMPs are provided in Appendix D1.

**Table 8-5
Pre-approved BMPs with Runoff Reduction Recognitions for Reducing
Imperviousness as Part of LID Approach**

BMP ^A	Runoff Reduction Recognition ^B
Porous Pavement	<p>Area covered by Porous Pavement with a minimum of 8 inch washed aggregate base may be assigned a weighted CN value of 87 (instead of CN of 98 normally used for impervious surfaces) for the purpose of Channel Protection Volume calculations. Use a weighted CN of 74 for the purpose of Water Quality Volume calculations, if needed.</p> <p>Note: If this BMP is specifically designed to provide permanent volume reduction through infiltration or through providing detention storage within the aggregate void, the volume reduction recognition discussed in Step 5 should be pursued instead of the CN reduction recognition.</p>
Vegetated Roof	<p>Vegetated roofs are designed to reduce runoff volumes. However, the volume reduction is highly dependent on the media and planting used, with the calculation methods very complex at times. In lieu of calculating the volume reduction benefits, the roof area with vegetated roof with a minimum media depth of 4 inches and a void ratio of 0.3 (as described in the fact sheet) may be assigned a weighted CN of 87 (instead of CN of 98 normally used for impervious surfaces) for the purpose of Channel Protection Volume calculations. Use a weighted CN of 74 for the purpose of Water Quality Volume calculations, if needed.</p>

Notes:

- A. In using and recognizing the runoff reduction of these BMPs, applicants must meet the review criteria located within the discussion of each BMP provided in Appendix D1.
 - B. If the LID track is pursued, reduced CNs for areas covered by these BMPs may be used for determining the post-developed runoff rates and volumes for larger events (up to and including the 100-year event). See **Table 8-9** for weighted CN values used for such larger events.
- Although imperviousness reduction BMPs are encouraged throughout any new development or re-development, the runoff reduction recognition may only be considered where the following conditions are met:

- The BMP must be in the common areas and covered by an easement or other agreement that assigns responsibility for its maintenance.
- The BMP must be covered by a maintenance plan and agreement with assurances for the long-term availability of maintenance funds (such as funds held in a permanent escrow account) provided to the City of Lafayette in a form acceptable to the City of Lafayette

4. **Calculate the amount of volume control needed for channel protection**

- Determine the 1-year 24-hour rainfall from Table 2-5.
- Delineate sub-basins in a manner that, at a minimum and to the extent possible, the pervious and impervious surfaces are in different sub-basins.
- Determine the disturbed drainage area for each sub-basin by subtracting the protected area determined in Step 1 from total contributing drainage area.
- Assign CN to each cover type and land use, assigning “credited CN” for areas treated in Steps 2 and 3 instead of normal post-development CN that is determined based on the proposed land use and the next less infiltrating underlying soil group, when applicable. Published pre-determined weighted CN values shall not be utilized for LID approach. This applies regardless of whether manual methods or computer modeling techniques are used.
- Determine the total post-development 1-year, 24-hour runoff volume for the entire site’s disturbed areas through the use of acceptable computer models or manually as specified below. This is the net Channel Protection volume needing to be permanently removed by appropriate structural BMPs.
 - Computer Model: Use acceptable computer models (listed in Chapter 2) to determine the total runoff volume for the site, utilizing 1-year, 24 hour rainfall depth with Soil Conservation Service (SCS) type 2 storm distribution, drainage area, and CN determined above, according to the Soil Conservation Service (SCS) CN loss method along with SCS unitless hydrograph methodology.
 - Manual Calculation: If calculating manually, use the following formula:
 - i. Runoff Volume (ft³) for each cover type = $Q_v \times 1/12 \times A$
 Where
 A = disturbed area of the particular cover type (ft²)
 Q_v = Runoff Depth (in) = $(P - 0.2S)^2 / (P + 0.8S)$
 P = 1-Year, 24 Hr Rainfall (in)
 S = $1000 / CN - 10$
 - ii. Sum the individual volumes to obtain the total post-development runoff volume for area to be managed.

5. **Provide Distributed Volume Reduction/Infiltration Practices**

- **Table 8-6** includes a list of the structural BMPs from potential BMPs that provide volume removal. Select and design structural BMPs that provide volume control to meet, when combined, the total net channel protection

volume determined in Step 4. Fact sheets for these and other relevant post-construction structural BMPs are provided in Appendix D.

Table 8-6
Pre-approved Structural BMPs with Permanent Volume Reduction
Recognitions for Channel Protection as Part of LID Approach

BMP ^A	Channel Protection Volume Runoff Reduction Recognition ^B
Infiltration Practices (Infiltration Basin, Subsurface Infiltration Bed, Infiltration Trench, and Dry Well)	Volume reduction is achieved by surface storage volume (if included in the design), subsurface volume (if included in the design), and infiltration volume as described in the fact sheet. If an underdrain has to be used due to soil conditions, no volume reduction is granted for the “infiltration volume” portion.
Bioretention	Volume reduction is achieved by surface storage volume, soil storage volume, and infiltration bed volume as described in the fact sheet.
Vegetated Swale	Volume reduction is achieved by surface storage volume (if included in the design through inclusion of check dams) and active infiltration volume during the storm (when infiltration is expressly designed for as a purpose) as described in the fact sheet.

Notes:

- A. In using and recognizing the runoff reduction of these BMPs, applicants must meet the review criteria located within the discussion of each BMP provided in Appendix D1.
- B. If the LID track is pursued, the volume reduction provided by these BMPs may be recognized towards determining the post-developed runoff rates and volumes for larger events (up to and including the 100-year event). See **Table 8-9** for extent of runoff reduction recognition allowed for such larger events.

- The volume reduction BMPs may not be successfully implemented in every situation. See “Applicability and Limitation” discussions in each Fact Sheet. In order to qualify for credit, the BMPs must meet all the following:
 - Be in the common areas and covered by an easement or other agreement that assigns responsibility for its maintenance.
 - Be covered by a maintenance plan and agreement with assurances for the long-term availability of maintenance funds (such as funds held in a permanent escrow account) provided to the City of Lafayette in a form acceptable to the City of Lafayette.
 - Be constructed on undisturbed A or well-drained B soils (B/D soils do not qualify) or amended soil with underdrains, as needed. If underdrains are used, the bottom elevations of the underdrains should be above the seasonal high water table. Soil infiltration testing protocol, provided in Appendix D4, must be followed to determine if infiltration BMPs are suitable at a site and to obtain the required data (such as soil conditions and depth of seasonal high water table) for infiltration design.

- Be constructed in an area where the depth of seasonal high water table and any bedrock is more than a minimum of 2 feet (4 feet is desirable) from ground elevation.
- Be constructed in a manner that any infiltration practices are adequately separated from basement foundations (50 feet up gradient, 10 feet down gradient), on-site septic systems/drainfields (100 feet), wells (50 feet), and other building elements that could be affected by infiltration systems.
- Be constructed outside of any 1-year (Zone 1) or 5-year (Zone 2) time of travel areas to public water supply wells, as defined by a modeled wellfield delineation performed in compliance with 327 IAC 8-4.1. When such delineation is not available, said practice must be at least 3,000 feet from the nearest public water supply well (unless applicant can demonstrate that the proposed practice will have no impacts on the water quality of the water supply well).
- Final construction should be completed after the contributing drainage area has been stabilized.
- Must contain erosion-protection features at the inflow to prevent scouring.
- Must contain a maintenance area near the inlet to collect large debris. Examples include small concrete aprons, catch basin inserts, or similar durable maintenance point.
- When the LID Approach is being pursued in all other aspects of the design but site limitations would not allow permanent volume reduction practices, channel protection volume shall, at a minimum, be accommodated through distributed storage solutions noted in Table 8-6 that also include underdrains as described in the appropriate Fact Sheets so that at a minimum they can act as both extended detention and filtration practices.
- **Calculation Methods for Recognizing Impacts of Distributed Storage on Overall Site's Peak Flow Detention Requirements:** When all the stated conditions above are met for volume-reduction distributed storage practices noted in Table 8-6, total volume provided for channel protection within distributed storage units (not to exceed the required channel protection volume calculated in Step 4) may be applied towards the site's overall detention requirements for peak (10-year and 100-year) flow control (see **Table 8-9**). Several methods are available to account for the noted runoff reduction recognition. A few common methods are listed below (other methods not noted below may also be used as appropriate):
 - Method 1: Assume that the provided Channel Protection Volume in the distributed storage units (not to exceed the required channel protection volume calculated in Step 4) will be stored below the detention pond's normal pool (below the lowest outlet). To simulate this condition, all the volumes in the elevation-storage table are increased by the provided Channel Protection Volume, an additional table entry is made as the first row with an artificial lower elevation and with zero for storage, and the reservoir's starting elevation is set at the noted artificial elevation.
 - Method 2: Utilize the "Divert" option of the hydrologic model used to compute the inflow to the pond to simulate the diversion (abstraction) of

the provided Channel Protection Volume (not to exceed the required channel protection volume calculated in Step 4) from the detention pond inflow before the remaining flood hydrograph is routed through the detention system. To accomplish this, the model should have capability to simulate diversion with a volume cap option.

- Method 3: Explicitly model the distributed storage features as a network of storage and conveyance units through the use of computer programs that can correctly model interconnected storage.

6. **Provide Additional (As-needed) Extended Detention Practices**

- When the LID Approach is being pursued in all other aspects of the design but site limitations would not allow adequate distributed volume reduction practices noted in Step 5 (with or without underdrains), then a constructed wetland or a wet-bottom extended detention facility along with incorporation of an appropriate wetland fringe should be utilized as listed in **Table 8-6**. If designed properly, such a facility can be incorporated into a multi-purpose facility to control channel protection volume, water quality volume, and 100-year peak flow rate. Note that since by design conditions of a wet-bottom extended detention facility, 90% of the original volume will be available within 48 hours of each storm event (i.e., a maximum of 36 hours from when the channel protection pool is full), the volume in the pond associated with the channel protection may be assumed empty for the purpose of peak flow detention analysis discussed in Chapter 6 of these Standards (see **Table 8-9**).

**Table 8-7
Pre-approved BMPs with Additional, As-needed Extended Detention Runoff
Reduction Recognitions for Channel Protection as Part of LID Approach**

BMP ^A	Runoff Reduction Recognition ^B
Constructed Wetland	The volume of the supplementary extended detention, in lieu of permanent volume reduction, is applied towards meeting Channel Protection Volume requirements so long as only 10% of the maximum stored volume is left in the basin after 36 hours from maximum storage time and no more than 40% from the maximum stored volume is released within the first 12 hours.
Extended Detention Wet/Dry Pond	The volume of the supplementary extended detention, in lieu of permanent volume reduction, is applied towards meeting Channel Protection Volume requirements so long as only 10% of the maximum stored volume is left in the basin after 36 hours from maximum storage time and no more than 40% from the maximum stored volume is released within the first 12 hours.

Notes:

- A. In using and recognizing the runoff reduction of these BMPs, applicants must meet the review criteria located within the discussion of each BMP provided in Appendix D1.
- B. If the LID track is pursued, the volume reduction provided by these BMPs may be applied towards determining the post-developed runoff rates and volumes for larger events (up to and including the 100-year event). See **Table 8-9** for extent of volume reduction allowed for such larger events.

7. If needed – determine water quality volume and select appropriate BMPs

A channel protection volume BMP can also be used as a post-construction stormwater quality BMP if it meets the design criteria in Appendix D. City of Lafayette has designated a number of pre-approved BMP methods to be combined in various ways to achieve the stormwater quality performance criteria noted in Section A of this chapter. BMP methods address the runoff generated from not more than the first inch of rainfall for the entire site (disturbed and undisturbed) tributary to each outlet. These BMP measures are listed along with their anticipated average pollutant removal rates in **Table 8-2**. Applicants desiring to use a different removal rate for these BMPs must follow the requirements for Innovative BMPs. Details regarding the applicability and design of these pre-approved BMPs, including the effectiveness of these BMPs in removing TSS, are contained within fact sheets presented in Appendix D.

The methodology to determine the design water quality volume or rate for the LID approach is the same as that described for the Channel Protection Volume Calculation described in Step 4, except the rainfall depth for water quality will be

1 inch instead of the 1-year, 24-hour rainfall depth used for calculating the Channel Protection Volume. A few considerations specific to water quality volume/rate calculations are as follows:

- Time of concentration in the case of LID design is the time it takes a drop of water to move from the furthest point in the disturbed area to its discharge from the disturbed area.
- If calculating manually, use the following formula:
 - Runoff Volume (ft³) = $Q_v \times 1/12 \times A$
Where
A = disturbed area of the particular cover type (ft²)
 $Q_v = \text{Runoff Depth (in)} = (P - 0.2S)^2 / (P + 0.8S)$
P = 1 inch
S = 1000/CN – 10
 - Peak Runoff Rate (ft³/sec) = $q_u \times A \times Q_v \times 1/43,560$
Where
A = disturbed area of the particular cover type (ft²)
 $Q_v = \text{Runoff Depth (in)}$ calculated in previous step
 $q_u = \text{Unit Peak Discharge (cfs/mi}^2/\text{in)}$, determined from TR-55 Exhibit 4-II
 - Sum the individual volumes and peak runoff rates to obtain the total design post-development water quality runoff volume and rate.

8. **Complete the LID Approach Utilization Summary Form**

- As the final step of the LID Approach, a summary of non-structural and structural BMPs utilized, as part of the LID Approach, in the site design of a particular development site is provided in **Table 8-9** and submitted as part of the permit request package.
- The presence of Table 8-9 in the submittal package and the information contained in the form would alert the plan reviewer that the LID Approach is being used to meet the post-construction stormwater quality requirements of the site and that the overall site design as well as peak discharge and detention calculations should be reviewed with the impacts of LID Approach in mind.

**Table 8-8
LID Approach Summary Worksheet**

Project Information				
Project Name		Developer		
Project Address		Developer's Address		
Engineer		Developer's Telephone		
<p>This worksheet is a tool to allow both the regulatory agency and the Developer to reference various LID measures implemented within the development in order to meet the development's Post Construction Stormwater Management requirements.</p>				
Site Specific Information				
Total Site Area:		_____ sf		
Proposed Earth Disturbance Area:		_____ sf		
Existing Impervious Area:		_____ sf		
LID Measure Category	Potential BMPs	Total Surface Area (sf) of LID Measure	Plan Pg # of LID Measure	Pg # of Calculations for LID Measure
Minimize Disturbed Areas	Protect Sensitive Areas			
	Protect Riparian Buffers			
	Protect Natural Flow Pathways			
	Minimize Total Disturbed Area			
	Reduce Impervious Surfaces			
	Cluster-Type Development			
Restore Disturbed Areas	Minimize Soil Compaction			
	Protect Trees in Disturbed Areas			
	Soil Amendment and/or Restoration			
	Native Revegetation			
	Riparian Buffer Restoration			
Minimize Imperviousness	Porous Pavement			
	Vegetated Roof			
Provide Distributed Retention/Infiltration Practices	Infiltration Practices*			
	Bio-retention			
	Vegetated Swale			
Total Surface Area of LID Measures (sf)				
Proposed Final Impervious Surface Area				_____ sf
Percent of Total Site Area Covered by LID				_____ %
<p>Note: Not all LID measures are necessary or appropriate for every site. It is imperative that proper site assessments and due diligence is completed by the Developer and/or Engineer prior to design.</p>				

*: Infiltration Practices include: Infiltration Basins, Subsurface Infiltration Beds or Trenches, and Dry Wells

Summary of Runoff Reduction Recognitions for Water Quality Volume, Channel Protection, and Peak Flow Control Detention Volume for LID approach

As discussed throughout this Section, to encourage LID approach for stormwater management, runoff reduction recognitions towards all three major stormwater management requirements, i.e., Water Quality, Channel Protection, and Peak Runoff Detention, are associated with various BMPS as noted through the above seven step process. These runoff reduction recognitions are summarized in **Table 8-9**.

**Table 8-9
Summary of Runoff Reduction Recognitions for Pre-Approved BMPs Used in the LID Approach**

Implementation Order	DESCRIPTION	POTENTIAL BMPs	REDUCTION RECOGNITION FOR POST-CONSTRUCTION WATER QUALITY CALCULATIONS		REDUCTION RECOGNITION FOR WATER QUANTITY (DETENTION AND STORM DRAIN) CALCULATIONS
			WATER QUALITY VOLUME	CHANNEL PROTECTION VOLUME	
1	Minimize Disturbed Areas	<ul style="list-style-type: none"> Protect Sensitive Areas Protect Riparian Buffers Minimize Total Disturbed Area Protect Natural Flow Pathways Reduce Impervious Surfaces Cluster-Type Development 	Full recognition through allowing to use "disturbed surface area" only for all calculations	Full recognition through allowing to use "disturbed surface area" only for all calculations	Full recognition through allowing CN for the undisturbed, protected area to be calculated based on pre-developed underlying soil types
2	Restore Disturbed Areas	<ul style="list-style-type: none"> Minimize Soil Compaction Protection of Existing Trees within disturbed areas (part of Minimize Total Disturbed Area) Soil Amendment and Restoration Native Revegetation Riparian Buffer Restoration 	Full recognition through allowing CN for the restored/protected area to be calculated based on pre-developed underlying soil types	Full recognition through allowing CN for the restored/protected area to be calculated based on pre-developed underlying soil types	Full recognition through allowing CN for the restored/protected area to be calculated based on pre-developed underlying soil types
3	Minimize Imperviousness	<ul style="list-style-type: none"> Porous Pavement 	Full recognition of perviousness through allowing CN for the application area to be calculated based on a pre-set value (74 instead of 98) AND full recognition of the stored volume (if provided for in the design) and WQ treatment, if designed as a true infiltration practice (no underdrain/ or extended 24-48 hrs release)	Partial (weighted) recognition of perviousness through allowing CN for the application area to be calculated based on a pre-set value (87 instead of 98) AND full recognition of the stored volume (if provided for in the design), if designed as a true infiltration practice (no underdrain/ or extended 24-48 hrs release)	Partial (weighted) recognition of perviousness through allowing CN for the application area to be calculated based on pre-set values (89 for 10-year and 90 for 100-year calculations instead of using 98) and full recognition of the stored volume (if provided for in the design), on a case by case basis, treated as an underground detention
		<ul style="list-style-type: none"> Vegetated Roof 	Full recognition of perviousness through allowing CN for the application area to be calculated based on a pre-set value (74 instead of 98) AND full recognition of the stored volume (if provided for in the design) and/or WQ treatment if designed for	Partial (weighted) recognition of perviousness through allowing CN for the application area to be calculated based on a pre-set value (87 instead of 98)	Partial (weighted) recognition of perviousness through allowing CN for the application area to be calculated based on a pre-set value 89 for 10-year and 90 for 100-year calculations instead of using 98)
4	Provide Distributed Volume Reduction/Infiltration Practices (or Filtration Practices, if underdrains have to be provided) in Common Areas	<ul style="list-style-type: none"> Infiltration Practices (Infiltration Basin, Subsurface Infiltration Bed, Infiltration Trench, and Dry Well) Bioretention Vegetated Swale 	Full recognition of perviousness through allowing CN for the application area to be calculated based on cover type and underlying soil AND full recognition of the retained volume (if provided for in the design) and/or WQ treatment if designed for	Full recognition of retained volume if designed as true infiltration practice (on appropriate soil and no underdrain/ or extended 24-48 hrs release)	Limited recognition of retained volume (up to the Channel Protection Volume) if designed as true infiltration practice (on appropriate soil and no underdrain/ or extended 24-48 hrs release)
5	Provide, As-needed, Additional Extended Detention Practices in Common Areas	<ul style="list-style-type: none"> Constructed Wetland Extended Detention Wet/Dry Pond 	full recognition of the stored volume (with extended 24-48 hrs release) and/or WQ treatment if designed for	Full Recognition of stored volume (with extended 24-48 hrs release)	Full Recognition of stored volume (with extended 24-48 hrs release)
6	Provide, As needed, Additional Water Quality BMPs	<ul style="list-style-type: none"> Pre-approved BMPs noted in Table 8-1 for conventional method 	Full recognition of WQ treatment	N/A	N/A

D. SPECIAL PROVISIONS FOR “HOT SPOT” LAND USES

For all those projects involving land uses considered to be high pollutant producers or “hot spots” (see **Table 8-10** e.g., vehicle service and maintenance facilities, vehicle salvage yards and recycling facilities, vehicle and equipment cleaning facilities, fleet storage areas for buses, trucks, etc., industrial/commercial or any hazardous waste storage areas or areas that generate such wastes, industrial sites, restaurants and convenience stores, any activity involving chemical mixing or loading/unloading, outdoor liquid container storage, public works storage areas, commercial container nurseries, and some high traffic retail uses characterized by frequent vehicle turnover), additional water quality requirements may be imposed by the City of Lafayette in addition to those included in water quality criteria in order to remove potential pollutant loadings from entering either groundwater or surface water systems. These pre-treatment requirements are included in **Tables 8-10 and 8-11**.

**Table 8-10
Pre-Treatment options for Stormwater Hot Spots**

Stormwater Hot Spots	Minimum Pre-Treatment Options
Vehicle Maintenance and Repair Facilities	A, E, F, G
Vehicle Fueling Stations	A, D, G
Drive-through Restaurants, Pharmacies, Convenience Stores	B, C, D, I, K
Outdoor Chemical Mixing or Handling	G, H
Outdoor Storage of Liquids	G
Commercial Nursery Operations	I, J, L
Other Uses or Activities Designated by Appropriate Authority	As Required

**Table 8-11
Minimum Pre-Treatment Options**

Minimum Pre-Treatment Options	
A	Oil/Water Separators / Hydrodynamic Separators
B	Sediment Traps/Catch Basin Sumps
C	Trash/Debris Collectors in Catch Basins
D	Water Quality Inserts for Inlets
E	Use of Drip Pans and/or Dry Sweep Material under Vehicles/Equipment
F	Use of Absorbent Devices to Reduce Liquid Releases
G	Spill Prevention and Response Program
H	Diversion of Stormwater away from Potential Contamination Areas
I	Vegetated Swales/Filter Strips
J	Constructed Wetlands
K	Stormwater Filters (Sand, Peat, Compost, etc.)
L	Stormwater Collection and Reuse (especially for irrigation)
M	BMPs that are a part of a Stormwater Pollution Prevention Plan (SWPPP) under a NPDES Permit

E. CONSTRUCTION SEQUENCING CONSIDERATIONS

BMPs noted in this chapter refer to post-construction BMPs, which continue to treat stormwater after construction has been completed and the site has been stabilized. Installing certain BMPs, such as bioretention areas and sand filters, prior to stabilization can cause failure of the measure due to clogging from sediment. If such BMPs are installed prior to site stabilization, they should be protected by traditional erosion control measures.

In those instances, the construction sequence must require that the pond is cleaned out with pertinent elevations and storage and treatment capacities reestablished as noted in the accepted stormwater management plan.

F. INSPECTION AND MAINTENANCE REQUIREMENTS

Subsequent to successful installation of Post-construction BMPs, they need to be inspected and maintained regularly in accordance with the Operation and Maintenance Manual required to be prepared for each BMP. An operations and maintenance (O&M) manual for all MS4-owned or private infrastructure, including but not limited to pipes, ponds, ditches, and BMPs (when required), shall be submitted for the final plan approval and permit process. The manual will become a maintenance guide for the drainage infrastructure once development is complete. The final O&M manual will be provided to the City of Lafayette in both hard copy and digital formats. The O&M manual, BMP maintenance agreement, along with a site map showing the BMP locations shall be recorded with the final plat. The O&M manual will include the following:

1. Name, address, business phone number, home phone number, email address, cellular phone number, pager number of owner;
2. Site drawings (8½" by 11" or 11" by 17"), showing both plan and cross-section views, showing the infrastructure and applicable features, including dimensions, easements, outlet works, forebays, signage, etc., as well as an overall site map of the development showing all structures;
3. Guidance on owner-required periodic inspections;
4. Requirement of owner to perform maintenance specified by City of Lafayette inspection, if any;
5. Guidance on routine maintenance, including mowing, litter removal, woody growth removal, signage, etc.;
6. Guidance on remedial maintenance; such as inlet replacement, outlet works maintenance, etc.;
7. Guidance on sediment and trash removal, both narrative and graphical, describing when sediment removal should occur in order to ensure that BMPs and other infrastructure remain effective as water quality and/or quantity control devices;
8. A statement that the City of Lafayette's representatives have the right to enter the property to inspect the infrastructure;
9. A tabular schedule showing inspection and maintenance requirements;
10. Identification of the property owner as the party responsible for all maintenance, including cost.
11. Identification of the Homeowners Association, if applicable, for eventual transfer of BMP ownership and maintenance responsibilities.

Inspection checklists for various types of BMPs are provided in Appendix D5. A sample of the required Stormwater Management Maintenance Agreement is provided in Appendix D6, with the required Stormwater Management Maintenance Agreement customized for City of Lafayette provided in Appendix B.1. This agreement will need to be further customized, signed, notarized, and recorded so that it can be a part of the property's deed. The maintenance agreement and the O&M Manual shall be

transferred to the new owner as the ownership and BMP maintenance responsibilities change hands.



Chapter Nine

METHODOLOGY FOR DETERMINATION OF REQUIRED SIZING OF BMPs

A. INTRODUCTION

This Chapter describes the acceptable methods for calculating Channel Protection Volume, Water Quality Volume, and Flow-Through BMP Flow Rate associated with Conventional Stormwater Management Approach (acceptable methods associated with the LID Stormwater Management Approach are discussed in Chapter 8). Channel Protection is achieved through retention or extended detention of runoff volume for 1-year, 24-hour storm event. Structural Water Quality treatment is achieved by treating the first inch of rainfall, either through retention/detention BMPs or by Flow-through BMPs. Detention/Retention BMPs impound (pond) the runoff to be treated, while flow-through BMPs treat the runoff through some form of filtration process.

B. RETENTION/DETENTION BMP SIZING

1. Channel Protection Volume

Channel Protection Detention/Retention BMPs must be designed to store the channel protection volume. The channel protection volume, CPv, is the storage needed to retain or detain the runoff to the receiving stream from the 1-year, 24-hour rainfall. The methodology for calculating the Channel Protection Volume (CPv) for each of site's final outlets using computer models or manual calculation is as follows:

1. Computer Model: Use acceptable computer models (listed in Chapter 2) to determine the total runoff volume for the site contributing to each site's outlet, utilizing 1-year, 24 hour rainfall depth with Soil Conservation Service (SCS) type 2 storm distribution, drainage area, and the composite CN calculated for the site, according to the Soil Conservation Service (SCS) CN loss method along with SCS unitless hydrograph methodology.
2. Manual Calculation: If calculating manually, use the following formula:
$$CPv \text{ (ft}^3\text{)} = Qv \times 1/12 \times A$$

Where

A = total post-construction site area contributory to each outlet (ft²)

Qv = Runoff Depth (in) = $(P - 0.2S)^2 / (P + 0.8S)$

P = 1-Year, 24 Hr Rainfall (in)

S = $1000 / CN - 10$

2. Water Quality Volume

Water Quality Detention BMPs must be designed to store the water quality volume for treatment. The water quality volume, WQv, is the storage needed to capture and treat the runoff from the first one inch of rainfall. The water quality volume is equivalent to one inch of rainfall multiplied by the volumetric runoff coefficient (Rv) multiplied by the site area.

A calculation methodology similar to that described for the channel protection volume may be utilized, except that the rainfall depth (P) will be equal to 1, instead of the 1-year, 24-hour depth.

Alternatively, a simpler methodology may be used for calculation of WQv as follows:

$$WQv = \frac{(P)(Rv)(A)}{12}$$

where:

WQv = water quality volume for each site's outlet (acre-feet)

P = 1 inch

Rv = volumetric runoff coefficient

A = area in acres

The volumetric runoff coefficient is a measure of imperviousness for the contributing area, and is calculated as:

$$Rv = 0.05 + 0.009(I)$$

Where:

I is the percent impervious cover

For example, a proposed commercial site will be designed to drain to three different outlets, with the following drainage areas and impervious percentages:

Subarea ID	On-site Contributing Area (acres)	Impervious Area %	Off-Site Contributing Area (acres)
A	7.5	80	0.0
B	4.3	75	0.0
C	6.0	77	0.0

Calculating the volumetric runoff coefficient for subareas A, B and C yields:

$$R_v(\text{subarea A}) = 0.05 + 0.009(80) = 0.77$$

$$R_v(\text{subarea B}) = 0.05 + 0.009(75) = 0.73$$

$$R_v(\text{subarea C}) = 0.05 + 0.009(77) = 0.74$$

The water quality volumes for these three areas are then calculated as:

$$WQ_v(\text{subarea A}) = (1'')(R_v)(A)/12 = 0.77(7.5)/12 = 0.48 \text{ acre-feet}$$

$$WQ_v(\text{subarea B}) = 0.73(4.3)/12 = 0.26 \text{ acre-feet}$$

$$WQ_v(\text{subarea C}) = 0.74(6.0)/12 = 0.37 \text{ acre-feet}$$

Note that this example assumed no off-site sources of discharge through the water quality BMPs. If there were significant sources of off-site runoff (sometimes called runoff for upstream areas draining to the site), the designer would have the option of bypassing off-site runoff around the on-site systems, or the detention BMP should be sized to treat the water quality volume for the entire contributing area, including off-site sources.

C. FLOW-THROUGH TYPE BMP SIZING

Flow-through BMPs are designed to treat runoff at a peak design flow rate through the system. Examples of flow through BMPs include catch basin inserts, sand filters, and grassed channels. Another flow through BMP which is gaining popularity is a dynamic separator. Dynamic separators are proprietary, and usually include an oil-water separation component.

The following procedure should be used to estimate peak discharges for flow through BMPs (adapted from Maryland, 2000). It relies on the volume of runoff computed using the Small Storm Hydrology Method (Pitt, 1994) and utilizes the NRCS, TR-55 Method.

Using the WQv methodology, a corresponding Curve Number (CN_{wq}) is computed utilizing the following equation:

$$CN_{wq} = \left[\frac{1000}{10 + 5P + 10Qa - 10\sqrt{Qa^2 + 1.25QaP}} \right]$$

where:

CN_{wq}	=	curve number for water quality storm event
P	=	1" (rainfall for water quality storm event)
Qa	=	runoff volume, in inches = 1"×Rv = Rv (inches)
Rv	=	volumetric runoff coefficient (see previous section)

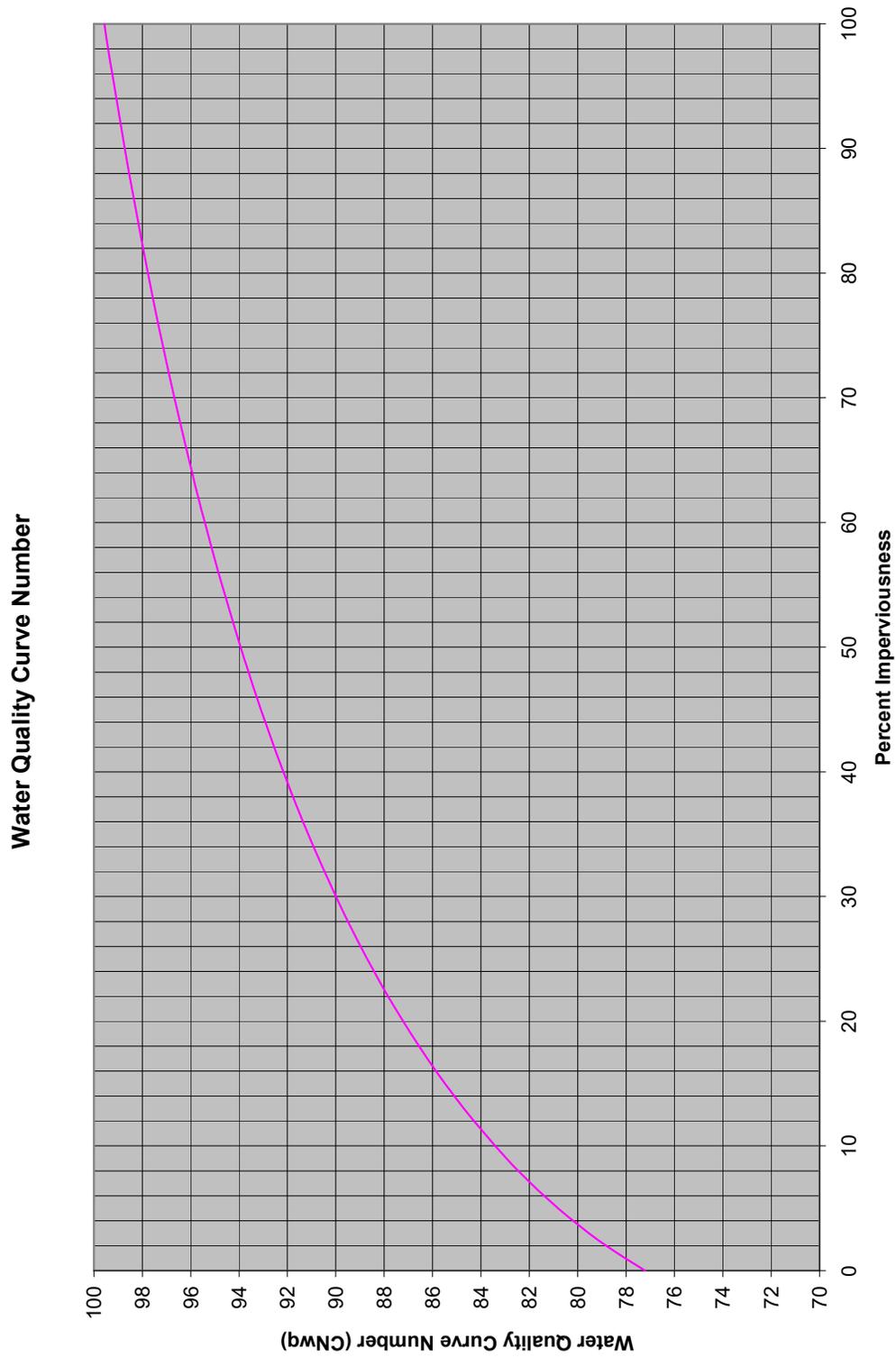
Due to the complexity of the above equation, the water quality curve number is represented as a function of percent imperviousness in **Figure 9-1**.

The water quality curve number, CN_{wq} , is then used in conjunction with the standard calculated time-of-concentration, t_c , and drainage area as the basis input for TR-55 calculations. Using the SCS Type II distribution for 1 inch of rainfall in 24-hours, the water quality treatment rate, Q_{wq} , can then be calculated.

REFERENCES

1. Maryland Stormwater Design Manual, Volume II, Appendix D.10, 2000
2. Pitt, R., 1994, Small Storm Hydrology. University of Alabama - Birmingham. Unpublished manuscript. Presented at design of stormwater quality management practices. Madison, WI, May 17-19 1994.
3. Schueler, T.R. and R.A. Claytor, 1996, Design of Stormwater Filter Systems. Center for Watershed Protection, Silver Spring, MD.
4. United States Department of Agriculture (USDA), 1986. Urban Hydrology for Small Watersheds. Soil Conservation Service, Engineering Division. Technical Release 55 (TR-55).

Figure 9-1
Curve Number Calculation for Water Quality Storm Event





Chapter Ten

MISCELLANEOUS REQUIREMENTS

A. POLICY ON DAMS AND LEVEES

Dams and levees have the potential for significant, sometimes catastrophic consequences should they fail. In order to minimize the potential for loss of life and public safety, decrease the potential for increased flood damage and disaster costs, and safeguard the downstream property rights, the following shall be required by the City of Lafayette and/or City of Lafayette Engineering for any proposed new or improvements to any existing dam or levee. These requirements are in addition to what is normally required for other development subject to the Ordinance or these Technical Standards and/or that required by State or Federal agencies.

1. Design of dams shall follow the requirements of the latest edition of IDNR-Division of Water “General Guidelines for New Dams and Improvements to Existing Dams in Indiana” as well as principles provided in the latest edition of “Indiana Dam Safety Inspection Manual”.
2. Design of levee/floodwalls shall follow the FEMA requirements and guidelines provided in 44 CFR Section 65.10 and USACE Engineer Manual 1110-2-193, Design and Construction of Levees.
3. An Incident and Emergency Action Plan (IEAP), including a detailed dam breach inundation map, shall be developed in accordance with the template provided in the latest edition of “Indiana Dam Safety Inspection Manual” and submitted to the City of Lafayette and/or City of Lafayette Engineering. The detailed dam breach inundation map referenced in this paragraph shall be developed for both “Sunny Day Breach” Scenario (breach during normal loading conditions) and for maximum loading condition with breach assumed to occur as the spillway system is passing the Spillway Design Flood (SDF) associated with the dam (“SDF + Breach” Scenario).
4. Unless the “Sunny Day Breach Inundation Area” is entirely contained within the applicant’s property and/ or contained within the existing 100-year floodplain, a copy of recorded flood/inundation easement or a recorded written consent for every property within the potential “Sunny Day Breach Inundation Area” shall be submitted to the City of Lafayette and/or City of Lafayette Engineering. In addition, all the affected property

owners whose properties are located within the “SDF + Breach Inundation Area” must be notified of a hearing relevant to the proposed added flooding risk, should such a man-made structure suffer a catastrophic failure. Notification of the time and place of the hearing shall be made in person or by certified mail at least five (5) to ten (10) days prior to the hearing. Proof of notice to each landowner shall be filed by affidavit with the City of Lafayette and/or City of Lafayette Engineering prior to the hearing.

5. A copy of a Management and Maintenance Plan for the proposed dam or levee developed in accordance with the latest edition of “Indiana Dam Safety Inspection Manual” shall be submitted to the City of Lafayette and/or City of Lafayette Engineering.
6. Unless the dam is subject to and regulated by IDNR, following the permitting and construction of the dam or levee, a copy of a formal periodic inspection report prepared in accordance with the recommendations contained in the latest edition of “Indiana Dam Safety Inspection Manual” shall be submitted to the City of Lafayette and/or City of Lafayette Engineering along with evidence that the identified maintenance deficiencies have been corrected. The inspection report has to be submitted as it gets completed in accordance with the inspection frequency recommended in the latest edition of “Indiana Dam Safety Inspection Manual”.

B. REQUIREMENTS ASSOCIATED WITH PROPOSED DEVELOPMENTS DOWNSTREAM OF DAMS

As indicated in the previous section, dams have the potential for significant, sometimes catastrophic consequences should they fail. Placing new development downstream of an existing dam does not only expose the future residents or users of the newly developed areas to a potential new significant risk, but could also have an impact on the hazard rating of the dam itself, which can in turn make the dam non-compliant with the state and federal standards. In order to minimize the potential for loss of life and public safety, decrease the potential for increased flood damage and disaster costs, and safeguard the upstream dam owner’s rights, the following shall be required by the City of Lafayette and/or City of Lafayette Engineering for any proposed new development or redevelopment downstream of an existing dam. These requirements are in addition to what is normally required for new development or redevelopment subject to these Standards.

1. Dam breach inundation maps have been created for several existing dams within the State of Indiana by the dam owners, IDNR, or others as part of development of individual IEAPs for these dams. When the development location is suspected by the applicant or the City of Lafayette and/or City

of Lafayette Engineering to be within an existing dam's breach inundation zone, the applicant is required to include a copy of the breach inundation mapping associated with that dam as part of its stormwater management permit application. To locate such a mapping, the applicant should contact the dam owner, IDNR- Division of Water, City of Lafayette and/or City of Lafayette Engineering, or other agencies to obtain the breach inundation map for the dam, if available. If no dam breach inundation map can be located for the dam, it will be the applicant's responsibility to produce a City of Lafayette and/or City of Lafayette Engineering-acceptable dam breach mapping either through directly contracting with a qualified engineer or through funding the production of such a map by the City of Lafayette and/or City of Lafayette Engineering review consultant in accordance with standards and guidelines established by the IDNR-Division of Water.

2. If the location of the proposed development falls within the dam breach inundation zone on the map discussed under Item 1 (above), additional requirements as determined by City of Lafayette and/or City of Lafayette Engineering may be imposed before a permit is issued and the development is allowed to occur. The noted additional requirements depend on several variables and are expected to vary case by case. Typical requirements could include relocating a portion or all of the proposed development to areas outside of the dam breach inundation zone, cost-sharing with the dam owner in necessary upgrades to the dam as a result of a potential hazard classification increase, addition of structural protection measures (such as flood protection levees), additional freeboard requirements, development and periodic exercise of warning and evacuation plans, and other measures considered necessary by City of Lafayette and/or City of Lafayette Engineering to minimize the potential for loss of life and public safety, decrease the potential for increased flood damage and disaster costs, and safeguard the upstream dam owner's rights.

C. FLOODPLAIN AVOIDANCE/NO NET LOSS FLOODPLAIN STORAGE POLICY

Floodplains exist adjacent to all natural and man-made streams, regardless of contributing drainage area or whether they have been previously identified or mapped. Due to potential impacts of floodplain loss on peak flows in streams and on the environment, floodplains are considered Impact Drainage Areas and disturbance to these areas should be avoided. When the avoidance of floodplain disturbance is not practical, the natural functions of the floodplain should be preserved to the extent possible.

In an attempt to strike a balance between the legitimate need for economic

development within the City of Lafayette jurisdictional area and the ever-increasing importance of and the need to preserve the natural functions of floodplains to the extent possible (especially given the ongoing and projected climate change impacts), compensatory excavation equivalent to the floodplain storage lost shall be required for all activities within area inundated by the 100-year flood elevation along floodplain of streams located in City of Lafayette where drainage area of the stream is equal to or larger than one square mile. The City of Lafayette Engineering may alter the compensation ratio, based on extenuating circumstances, for a specific project.

General Requirements

Note that by definition, compensatory storage is the replacement of the existing floodplain and, in rare exceptions, the floodway storage lost due to fill. Compensatory storage is required when a portion of the floodplain is filled, occupied by a structure, or when as a result of a project a change in the channel hydraulics occurs that reduces the existing available floodplain storage. Compensatory storage must:

- Be provided regardless of whether the flooding source is mapped or whether flood elevations are published or not. When flood elevations are not available for a flooding source that has a drainage area equal to or larger than one (1) square miles (640 acres), the applicant is to determine the 10-year, 50-year, and 100-year flood elevations at the site and get them approved by the IDNR prior to use for floodplain compensation calculations.
- Equal at least 1 times the volume of flood storage lost below the 10-year flood elevation, between 10- and 50-year flood elevations, and between 50-year and 100-year flood elevations;
- Be operational prior to placement of fill, structures, or other materials temporarily or permanently placed in the regulatory floodplain;
- Be provided along the stream within the property limits or at a location approved by the City of Lafayette;
- Be provided in such a way to mimic as close as possible the function provided by the lost floodplain storage. If the floodplain storage is to be lost outside the active flow conveyance path, then it must be compensated for outside the flow conveyance path (e.g., a flood conveyance shelf/2-stage ditch, while improving conveyance and erosion, is not an appropriate compensation for floodplain storage lost in the floodway fringe area).
- Be provided in addition to the site retention/detention volume; and
- Drain freely and openly to the waterway.

Compensatory storage is also required to be provided incrementally such that:

- All floodplain storage/conveyance capacity lost within the floodway shall be compensated for within the floodway;
- All floodplain storage lost within the floodway fringe shall be compensated for within the floodway fringe;
- All floodplain storage lost below the existing 10-year flood elevation shall be compensated for below the proposed 10-year flood elevation; and
- All floodplain storage lost between 10-year and 50-year flood elevations shall be compensated for between proposed 10-year and 50-year flood elevations.
- All floodplain storage lost between 50-year and 100-year flood elevations shall be compensated for between proposed 50-year and 100-year flood elevations.

Note that compensatory storage is required for activities in the regulatory floodplain. There is no threshold to compensatory storage; any volume of fill requires compensatory storage be provided. However, the compensatory storage requirement does not apply to specific activities in the regulatory floodplain, such as the floodproofing of an existing building, where the floodproofing measures such as berms or floodwalls are within 10 feet of the building, or crossing improvements, where artificially created storage is lost due to a reduction in head loss.

Computing Compensatory Storage

Computations must show 1 times compensation for floodplain storage volume lost for 10-year, 50-year, and 100-year storm events. Storage lost between the existing ground and the existing 10-year flood elevation must be compensated by providing 1 time the amount lost and be placed between the existing ground elevation and the proposed 10-year floodplain elevation. Storage lost between the existing 10-year and the existing 50-year flood elevation must be compensated by providing 1 times the amount lost and be placed between the proposed 10-year elevation and proposed 50-year flood elevation. Storage lost between the existing 50-year and the existing 100-year elevation must be compensated by providing 1 times the amount lost and be placed between the proposed 50-year elevation and proposed 100-year elevation.

When preparing a grading plan, thought should be given to how compensatory storage will be quantified. The most common methodology is the use of cross sections and the “average end area method”. The following requirements should be followed when preparing cross sections:

1. Prepare a detailed topographic survey tied to North American Vertical Datum of 1988 and the Tippecanoe County Survey Control Network benchmarks.

2. Locate cross sections parallel to each other and perpendicular to a reference line, often times a property line or fence line. Cross sections used in a hydraulic model are always perpendicular to flood flows, and not always parallel to each other. Therefore, these are often not suitable for computing flood fringe compensatory storage volumes.
3. Plot cross sections at a standard engineering scale so as to allow the reviewer to verify areas. Horizontal scale should be a maximum of 1"=50' and vertical scale should be a maximum of 1"=5', or as approved by the County.
4. Show existing grades, proposed grades, existing and proposed 10-year flood elevations, existing and proposed 50-year flood elevations, existing and proposed 100-year flood elevations, normal water level, a reference line, and floodway limits on the cross sections on the plans.
5. Locate cross sections no more than 150 feet apart, with a minimum of three cross sections per cut/fill area, or as necessary to accurately quantify cuts and fills.
6. Locate cross sections to pick up critical features such as berms, ditches, and existing and proposed structures.
7. Each cross section should be numbered or lettered and referenced on the plans.

This information is then utilized to compute the areas of cut and fill. A sample grading plan, a typical cross section, and associated compensatory storage calculations for the 10-year flood are provided on **Figures 10-1**, **Figure 10-2**, and **Table 10-1**, respectively.

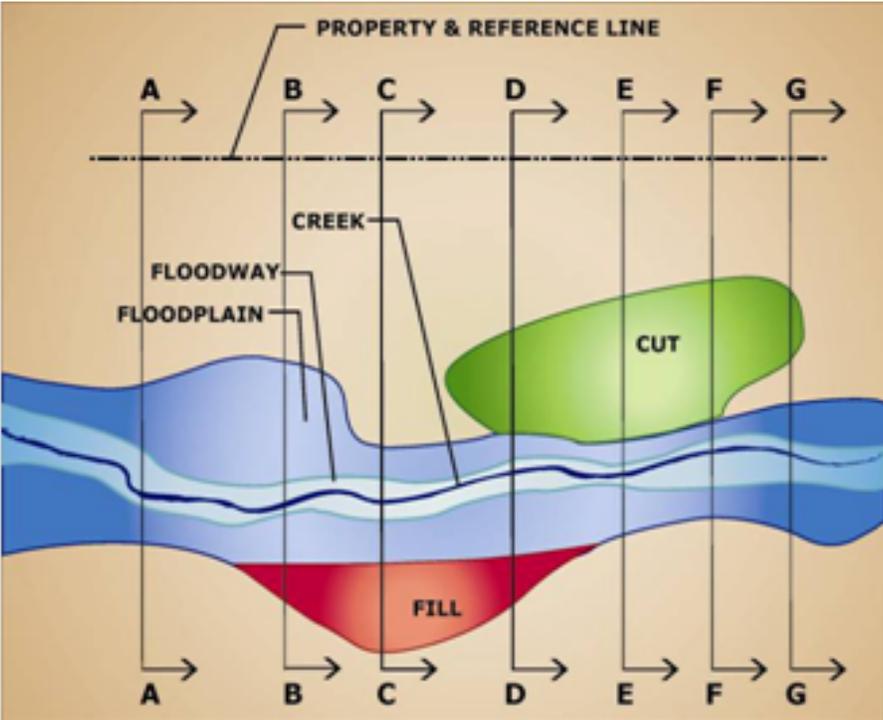
Volume of Fill between cross sections are calculated by finding the average fill cross sectional area and multiplying it by the distance between the two cross sections. For example, the fill volume between cross sections A and B is calculated as follows:

$$\begin{aligned} \text{Average Fill Area} &= (\text{Fill Area "A"} + \text{Fill Area "B"})/2 = (0 \text{ ft}^2 + 100 \text{ ft}^2)/2 = 50 \text{ ft}^2 \\ \text{Volume of Fill} &= (\text{Average Fill Area}) \times (\text{Distance}) = (50 \text{ ft}^2) \times (150 \text{ ft}) = 7,500 \text{ ft}^3 \end{aligned}$$

Once the total volume of fill placed, for this example, between the 0-and 10-yr flood elevations is determined, the total required compensatory storage can be calculated and compared against the total compensatory storage volume provided by the design as shown in the table. For this example:

$$\begin{aligned} \text{Required Compensatory Storage} &= (1) \times (\text{Total Volume of Fill}) = (1) \times (36,250 \text{ ft}^3) \\ &= 36,250 \text{ ft}^3 \end{aligned}$$

Figure 10-1 - Example Compensatory Storage Grading Plan



* Not to Scale & Topography not shown for clarity.

Figure 10-2 – Example Cross Section D-D

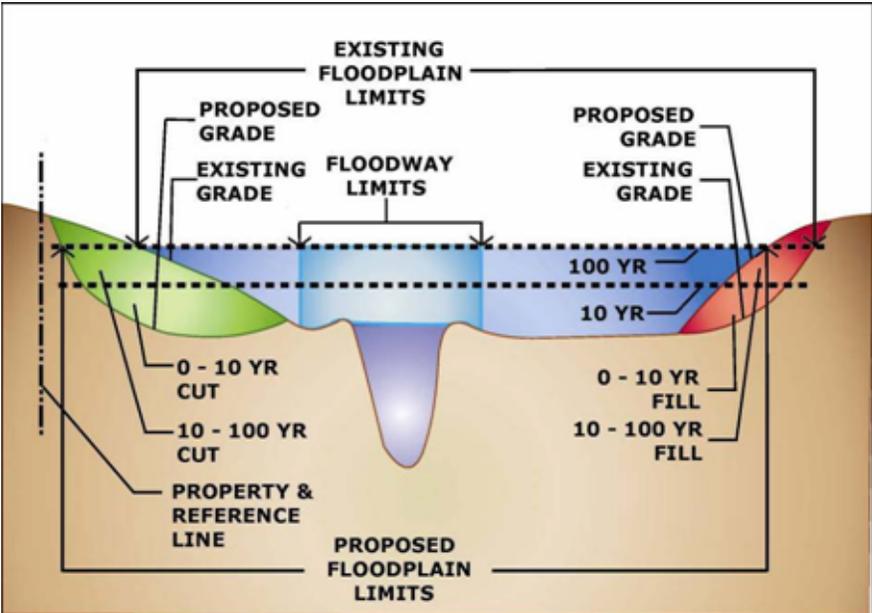


Table 10-1 - Example Compensatory Storage Calculations for 0-10 year event

Cross Section	Distance Between Sections (ft.)	Fill Area (sq. ft.)	Average Fill Area (sq. ft.)	Volume of Fill (cu. Ft.)	Cut Area (sq. ft.)	Average Cut (sq. ft.)	Volume of Cut (cu. Ft.)
A		0			0		
	150		50	7,500		0	
B		100			0		
	90		125	11,250		20	1,800
C		150			40		
	100		125	12,500		65	6,500
D		100			90		
	100		50	5,000		100	10,000
E		0			110		
	100		0	0		120	12,000
F		0			130		
	85		0	0		85	7,225
G		0			40		
Total Fill				36,250	Total Cut		37,525

Since the total amount of cut provided (37,525 ft^3 as shown in the table) is larger than that required (36,250 ft^3), the design meets the compensatory storage requirement for the 10-year flood. Additional tables and calculations should be completed for the flood storage between the 10-year and 50-year flood elevations and the flood storage between 100-year flood elevations in a similar manner to determine whether the design meets the compensatory storage requirement for the 100-year flood.

Location of Compensatory Storage

Compensatory storage must be located on-site and adjacent to or opposite the areas filled or occupied by a structure. In those rare instances when compensatory storage cannot be located adjacent to or opposite to the areas filled or occupied, engineering computations demonstrating that hydraulically equivalent compensatory storage has been provided is required. These computations must show that no increase in flood flows or flood depths will result as a result of the location of the proposed compensatory storage.

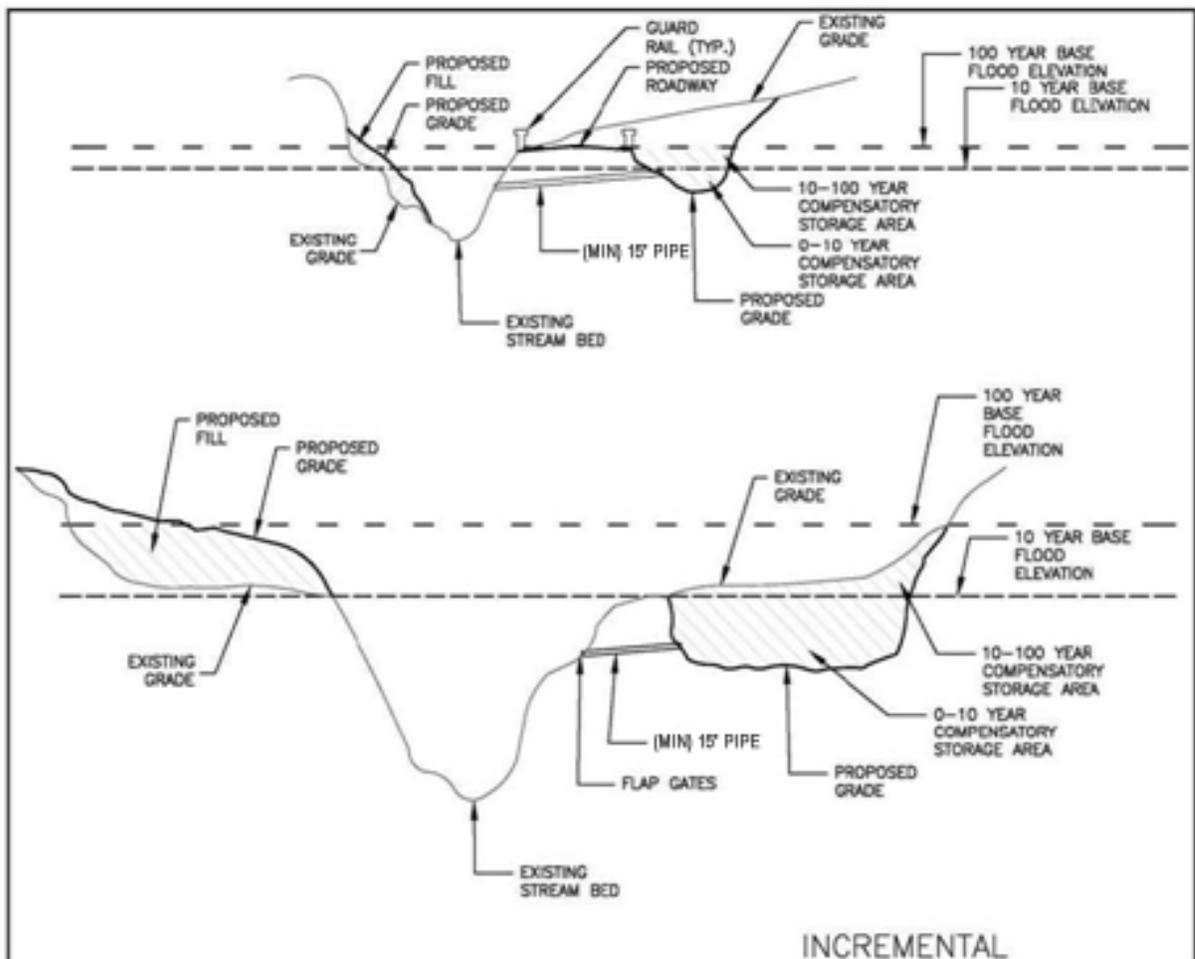
Compensatory storage must be constructed to drain freely and openly to watercourses. In some rare cases it may be necessary to install pipes to construct and/or operate a compensatory storage basin. This may occur when site constraints, such as a roadway or sidewalk, separate the waterway from the compensatory storage area. This is illustrated in the top half of **Figure 10-3**.

Another scenario may occur when a site cannot meet the incremental storage requirements discussed in this document. If incremental storage requirements from the 10-year to 50-year or from 50-year to 100-year elevations cannot be met, pipes could be installed with a flap gate to prevent the water from entering from the stream bed at lower elevations. The berm could then be set at the

elevation of the 10-year flood elevation, thus allowing the storage to only become effective above the 10-year flood elevation. This is illustrated in the bottom half of the illustration in Figure 10-3.

The use of pipes in compensatory storage will require approval by the County. Pipes must be a minimum of 15 inches in diameter so as to allow water to enter and exit freely with a minimum head differential. If the compensatory storage is proposed to be combined with detention, it must be demonstrated the compensatory storage and detention do not interfere with one another.

Figure 10-3 – Example of Compensatory Storage Connection to Stream through Pipe



Compensatory Storage in the Regulatory Floodway

Only fill associated with appropriate uses of the regulatory floodway may be allowed to be placed within the limits of the floodway. When in rare circumstances, fill is allowed, all provisions discussed above relating to compensatory storage must be met in addition to the items discussed below.

- Any fill placed within the existing floodway must be compensated for within the proposed floodway incrementally in the same manner discussed earlier for floodway fringe areas.

Should excavation within a floodway be allowed in special circumstances, there shall be no reduction in floodway surface area as a result of a floodway modification, unless such modification is part of a stream-wide plan or necessary to reduce flooding at an existing structure.

D. Development within Floodways and Fluvial Erosion Hazard Corridors

Regulatory Floodway and Fluvial Erosion Hazard (FEH) Corridor along regulated drains or any natural stream watercourse are considered Impact Drainage Areas and must be preserved to the extent possible to minimize flooding and erosion impacts within the stream system.

Basis for Establishing Floodways

The regulatory floodway along Tippecanoe County streams are established as detailed in the latest edition of the Tippecanoe County Floodplain Management Ordinance.

Basis for Establishing Fluvial Erosion Hazard (FEH) Corridors

The Indiana Silver Jackets Hazard Mitigation Task Force has initiated a multi-agency program to identify, study and provide mitigation planning resources for communities who would like to adopt Fluvial Erosion Hazard (FEH) avoidance strategies. The resources provided by this project enable individuals and communities to better recognize areas prone to natural stream-erosion processes and adopt strategies to avoid FEH-related risks. The FEH resources define approximate setbacks for communities to better manage river corridors. The setbacks vary based on the stream's recent migration history (actively migrating or relatively stationary).

For actively migrating and relatively stationary streams, a GIS analysis algorithm generated bankfull width values for each stream segment using regional curves that are based on drainage area within each physiographic region in Indiana. For relatively stationary streams, the analysis used these values to create buffer zones of at least one bankfull width on each side (a total corridor width of 3 times bankfull width) or 100 feet on each side of the bankfull bank, whichever is greater. For actively migrating streams, GIS generated a total corridor width of 8 times bankfull width, which was manually edited and refined to reflect the digital elevation model and stream meander evidence.

It should be noted that the FEH corridors were created at a map scale of approximately 1:10,000 to 1:15,000. The depicted areas were not meant to be accurate beyond providing an approximate boundary of potential stream

migration. A data layer containing FEH Corridors along Indiana streams is currently hosted by IDNR-Division of Water on their mapping portal currently at: <https://indnr.maps.arcgis.com/apps/webappviewer/index.html?id=43e7b307a0184c7c851b5068941e2e23>. The IDNR website notes that, due to its level of detail, this data is not intended for use in project design or parcel level site analysis and that a more detailed analysis of the localized geology and fluvial mechanics is necessary for the proper evaluation of the fluvial erosion hazard.

Despite its inherent level of accuracy, the FEH corridor map created by the Indiana Silver Jackets constitutes the best available data in Tippecanoe County and is the basis for regulating development within these corridors by City of Lafayette. A scaled version of the Tippecanoe County FEH corridor map is available from the Tippecanoe County GIS website. More detailed mapping than that used as part of the Indiana Silver Jackets erosion hazard mapping program may be provided by the applicant if it is based on detailed field assessment acceptable to the City of Lafayette.

Special Requirements within Floodways and FEH Corridors

No disturbance (fill or excavation) associated with a new development or redevelopment is permitted within FEH corridors or regulatory floodways, unless such modifications are part of a stream-wide restoration plan or a watershed master plan. This restriction does not apply to stormwater or tile outfalls. Any streambank stabilization effort within an FEH corridor shall require prior review by the City of Lafayette Engineering to safeguard against potential negative impacts on other properties or on the stream morphological system. Note that if a stream that is located within incorporated area of a community is designated as a County regulated drain, the proposed activities in the FEH corridor shall be under the County's jurisdiction.

E. Additional Consideration for Development Within or Adjacent to Bluff Zones

In addition to riverine erosion hazards that exist due to lateral migration of streams as discussed in Section D (above), there are hazardous areas along many bluffs existing in City of Lafayette. For the purpose of these Standards, Bluff is defined as a natural topographic feature having:

- a. A slope that rises at least 25 feet and the grade of the slope averages 30 percent or greater, measured over a horizontal distance of 25 feet, from the toe of the slope to the top of the slope. Where the slope begins below the ordinary high water level, the ordinary high water level is the toe of the slope. See Figure 10-4; or
- b. A natural escarpment or cliff with a slope that rises at least ten feet above the ordinary high water level or toe of the slope, whichever is applicable,

to the top of the slope, with a slope of 75 degrees or greater. See Figure 10-5.

These bluffs exist in many places and do not have to be necessarily along a permanent stream. They may be along the rivers (but sometimes outside the floodway and FEH zones) or along many headcut channels and developing ravines in City of Lafayette that may be tributary to various streams. To prevent buildings from damage due to natural slope adjustment next to bluffs, a minimum of 40 feet of landward setback shall be required from top of the bluff as illustrated in Figures 10-4 and 10-5. If a floodway or FEH is also present on the site, the most landward limit shall govern.

The definition of other terms shown in Figures 10-1 and 10-2 are as follows:

- Bluff impact zone: A bluff and land located within 20 feet of the bluff.
- Bluffline: A line delineating the top of the bluff. More than one bluffline may be encountered proceeding landward from the river.
- Toe of Bluff: A line along the bottom of a bluff, requiring field verification, such that the slope above the line exceeds 30 percent and the slope below the line is 30 percent or less, measured over a horizontal distance of 25 feet.
- Top of Bluff: A line along the top of a bluff, requiring field verification, such that the slope below the line exceeds 30 percent and the slope above the line is 30 percent or less, measured over a horizontal distance of 25 feet.

Where principal structures exist on the adjoining lots on both sides of a proposed building site, the minimum setback may be altered to conform to the average of the adjoining setbacks.

In addition to structure setback noted above, with the exception of erosion control practices sanctioned by or approved by the City of Lafayette Engineering, no land disturbance (fill or excavation) shall be allowed within a Bluff Impact Zone.

Also note that the minimum setback required in these standards is just that: a minimum, which is based on national experience. When buildings or other infrastructure is proposed near bluffs or steep slopes, the applicant is encouraged to seek a detailed geotechnical evaluation that may result in needing to have a larger setback or have special provisions even when the slopes are gentler than that regulated in these Standards.

Figure 10-4
Bluff and Bluff Impact Zone

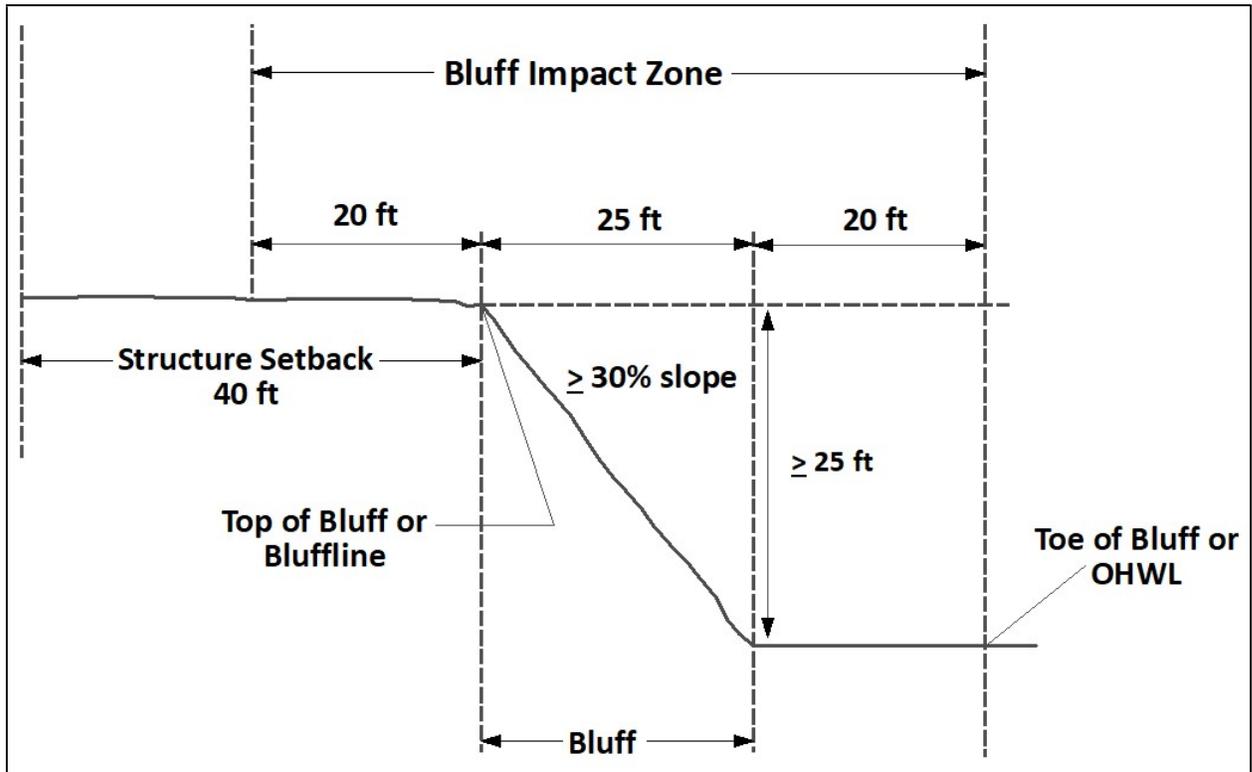
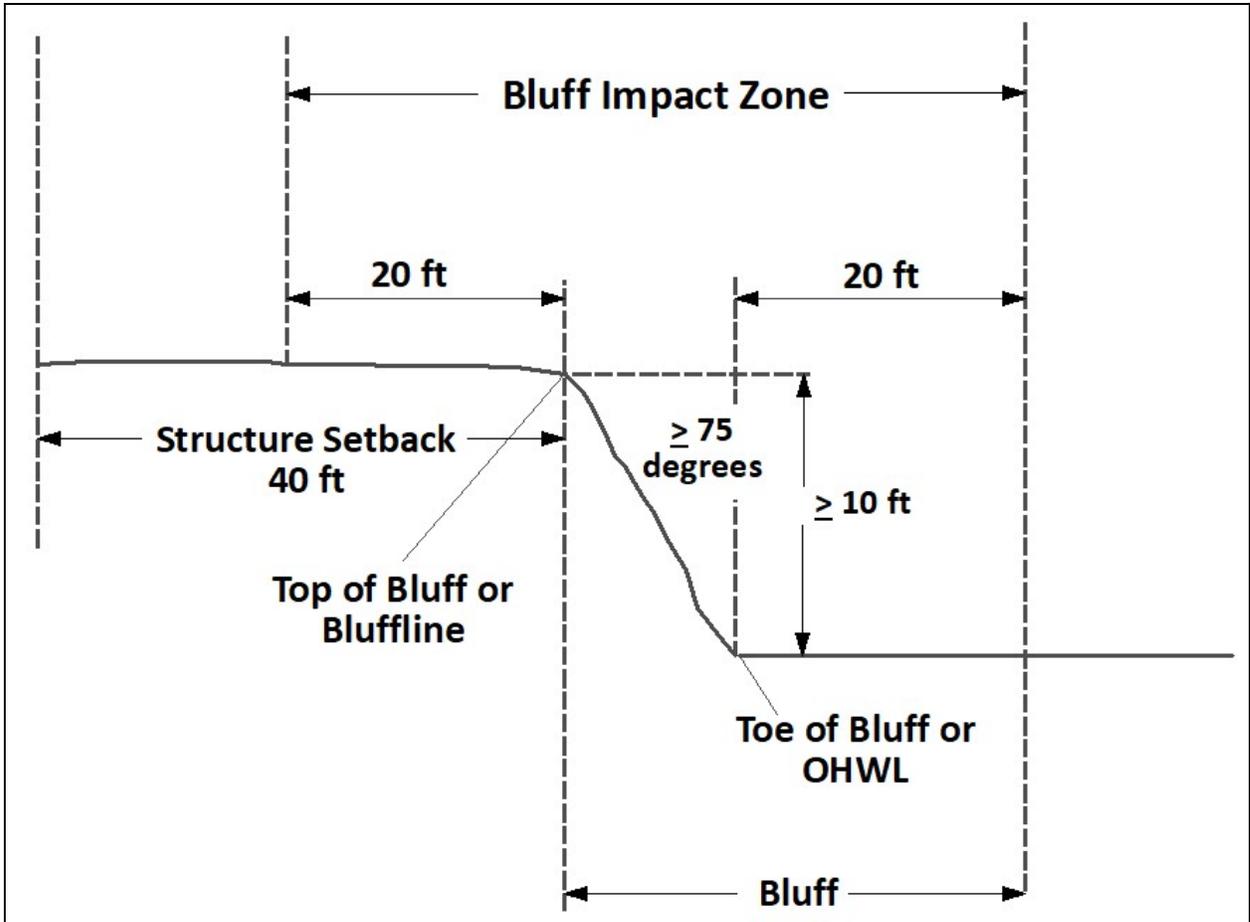


Figure 10-5
Natural Escarpment Bluff and Bluff Impact Zone



F. REQUIREMENTS ASSOCIATED WITH PROPOSED SOLAR FARMS

Solar development has expanded over the last several years as Indiana and other states have invested in this important resource to further greenhouse gas emission reductions. The large amount of impervious surface inherent in the construction of a large-scale solar array entails challenges not encountered in traditional development projects. If not properly managed through appropriate design and mitigation measures, stormwater discharged during and after the construction of solar arrays can be a significant source of pollution resulting from increased runoff, erosion, and sedimentation, which can adversely impact adjoining properties, streams, wetlands, or other natural resources. Solar installations must be properly designed to assure soil stabilization, minimize soil disturbance and soil compaction, and address ineffective controls to manage the total runoff volume and velocity that can lead to the loss of topsoil, erosion and sediment discharges from disturbed areas and stormwater outlets, and erosion along downstream channels and streambanks. The ability to address such significant environmental problems during construction and post-construction becomes more difficult as site imperviousness increases.

Solar projects that use traditional elevated solar panels are unique because they contain an impervious surface (elevated solar panel) that often has a pervious surface (vegetation) underneath the panel. Stormwater runoff from solar projects is generated primarily from rain that falls on access roads, inverter pads, and solar panels. Water that falls off solar panels runs across the panel to the dripline, and eventually falls to the underlying surface. Some of this water will infiltrate and some will run-off downslope and eventually off site.

One of the most notable impacts that solar sites have on water quality is the potential for erosion and/or scour at the dripline. To minimize the erosion and/or scour at the dripline, the lowest vertical clearance of any solar array shall be no greater than 10 feet. Also, erosion prevention and sediment control Best Management Practices (BMPs) as detailed in Chapter 7 of these Standards must be utilized during construction. The Tippecanoe County Unified Zoning Ordinance should be consulted to confirm any other height restrictions.

In addition to providing construction BMPs, Solar projects must adhere to the post-construction stormwater management requirements, including providing the required Water Quality Volume (WQv) and Channel Protection Volume (CPv) described in Chapter 8 of these Standards, as well as peak flow control (detention) requirements described in Chapter 6. However, because solar farms—particularly the panels— have unique characteristics, not like constructing a building or road, they often inherently include stormwater disconnection features that qualifies them for recognition/credit afforded to the Stormwater Disconnection BMP, which similar to typical solar panel farms rely on maintaining sheet flow and infiltration in adequately-sized, vegetative areas receiving runoff. The Stormwater Disconnection BMP is detailed in Appendix D1.

Per recommendations from the Center for Watershed Protection, the following Stormwater Disconnection recognitions/credits are established as part of these standards for solar farms that meet the conditions stated in these standards:

- i. WQv: Up to 100% of the required Water Quality Volume, proportionate to the percentage of total disconnected area to total site impervious area, may be subtracted from the required WQv.
- ii. CPv: For determining the Channel Protection Volume (CPv), the post-construction CN for the impervious area treated by the stormwater disconnection BMP may be determined assuming the treated area is “wood in good condition” (for the next less infiltrating hydrologic soil group than the pre-construction hydrologic soil group, since the area underneath panels is assumed disturbed/compacted during construction).
- iii. Qp (2, 10, 100): For determining the peak flow controls (detention), the post-construction CN for the area treated by the stormwater disconnection BMP, needed for determining post-construction peak flows (Qp) for 2, 10, and 100-year storms, may be determined assuming the treated area is “wood in good condition” (for the next less infiltrating hydrologic soil group than the pre-construction hydrologic soil group, since the area underneath panels is assumed disturbed/compacted during construction).

In order for the solar farm developments to be eligible for Stormwater Disconnection BMP recognitions/credits, the following design and construction guidelines must be met (Items d and g are required for all proposed solar farms regardless of whether Stormwater Disconnection recognition/credit is being sought):

- i. Roadways, gravel surfaces, transformer pads, and level spreaders within the solar field are considered effective impervious cover for the purposes of calculating Water Quality Volume (WQv), Channel Protection Volume (CPv), and post-construction peak flows.
- ii. All solar panels in the array should also be considered additional effective impervious cover for the purposes of calculating the WQv, CPv, and post-construction peak flow unless ALL the following conditions are met:
 - a) The vegetated area receiving runoff between rows of solar panels is equal to or greater than the average width of the row of solar panels draining to the vegetated area.
 - b) Overall site conditions and solar panel configuration within the array are designed and constructed such that the runoff remains as sheet flow across the entire site. Design array to ensure a perpendicular layout of drip edge to slope direction or install devices such as a

Level Spreader to ensure sheet flow from the drip edge. Level Spreaders shall be designed in accordance with the Level Spreader fact sheet contained in Appendix D1.

- c) The following conditions are satisfied regarding the design of the post-construction slope of the site:
- For slopes less than or equal to 5%, appropriate vegetation shall be established as indicated in **Figure 10-6**, below.
 - For slopes greater than 5%, but less than 10%, practices including, but not limited to, the use of level spreaders, infiltration trenches, or similar energy dissipating practices as described in **Figure 10-7**, below, shall be used to ensure long term sheet flow conditions.
 - For slopes equal to or greater than 10% and less than 15%, the Plan includes specific engineered stormwater control measures, such as level spreaders, infiltration trenches, or similar energy dissipating practices, with detailed specifications that are designed to provide permanent stabilization and non-erosive conveyance of runoff to the property line of the site or downgradient from the site.
 - Slopes greater than 15% are not qualified for a stormwater disconnection recognition/credit.
- d) The lowest vertical clearance of the solar panels above the ground should not be greater than ten (10) feet. The panels should, however, be at an adequate height to support vegetative growth and maintenance beneath and between the panels. If the lowest vertical clearance of the solar panels above the ground is greater than ten (10) feet, non-vegetative control measures will be necessary to prevent/control erosion and scour along the drip line or otherwise provide energy dissipation from water running off the panels. The Tippecanoe County Unified Zoning Ordinance should be consulted to confirm any other height restrictions.
- e) Disconnecting impervious surfaces works best in undisturbed soils. To minimize disturbance and compaction, construction vehicles and equipment should avoid areas used for disconnection during installation of the solar panels. Hydrologic Soil Group D soils or soils that are compacted by construction equipment may need to be tilled to a depth of four to six inches and/or amended to increase permeability.

- f) Groundcover vegetation must be maintained in good condition in those areas receiving disconnected runoff. Areas receiving runoff should be protected (e.g., planting shrubs or trees along the perimeter) from future compaction. Vegetated areas shall not be subject to chemical fertilization or herbicides/pesticides except for those applications that are necessary to get vegetation established and which follow an approved Erosion and Sediment Pollution Control Plan.

To maximize the potential for infiltration and reduce maintenance, the use of native deep-rooted vegetative cover under the panels and between the panel rows is highly encouraged. To achieve a native deep-rooted vegetative cover, a mixture of perennial grasses and wildflowers is recommended with a diversity of forbs or flowering plants that bloom throughout the growing season. Blooming shrubs may also be used in buffer areas as appropriate for visual screening. Perennial vegetation (grasses and forbs) should be native to Indiana, but where appropriate to the vegetative management plan goals, may also include other naturalized and non-invasive species which provide habitat for pollinators and wildlife and/or other ecosystem services.

- g) A fifty (50) foot buffer should be maintained between any part of the solar array and any “watercourse” or “waterbody) as that term is defined in Appendix A of these standards. The buffer shall consist of undisturbed existing vegetation or native shrub plantings.
- h) Similar to other post-construction BMPs, the vegetated area underneath the panels, the vegetated area receiving runoff, and any buffer areas will need to be mapped, maintained in accordance with the stormwater Management operation and maintenance manual, and covered by the recorded maintenance agreement described in Chapter 8 of these standards.

Depending on the layout and number of panels installed, the stormwater disconnection BMP may address some or all of the stormwater management requirements (WQv) for an individual project. Where the imperviousness is high or there is other infrastructure (e.g., access roads, transformers), additional runoff may need to be treated. Further reduction in the remaining required Water Quality, Channel Protection, and Peak flow control volumes is possible through utilizing the BMPs described as part of the LID track in Chapter 8 of these standards.

A solar panel project should ideally be installed and placed outside of the floodplain or detention facilities. If proposed to be placed within a floodplain or in a dry detention facility, panels (all tilt positions) must be installed at or above the flood protection grade (2 feet above the BFE) or at or above the 100-year emergency overflow of the detention facility plus one foot. This includes all

electrical systems associated with the panels. If the solar array project is proposed within floodway portion of the floodplain, the project shall also require a Construction in Floodway Permit from the IDNR.

Figure 10-6
Typical Solar Panel Installation with Slopes $\leq 5\%$

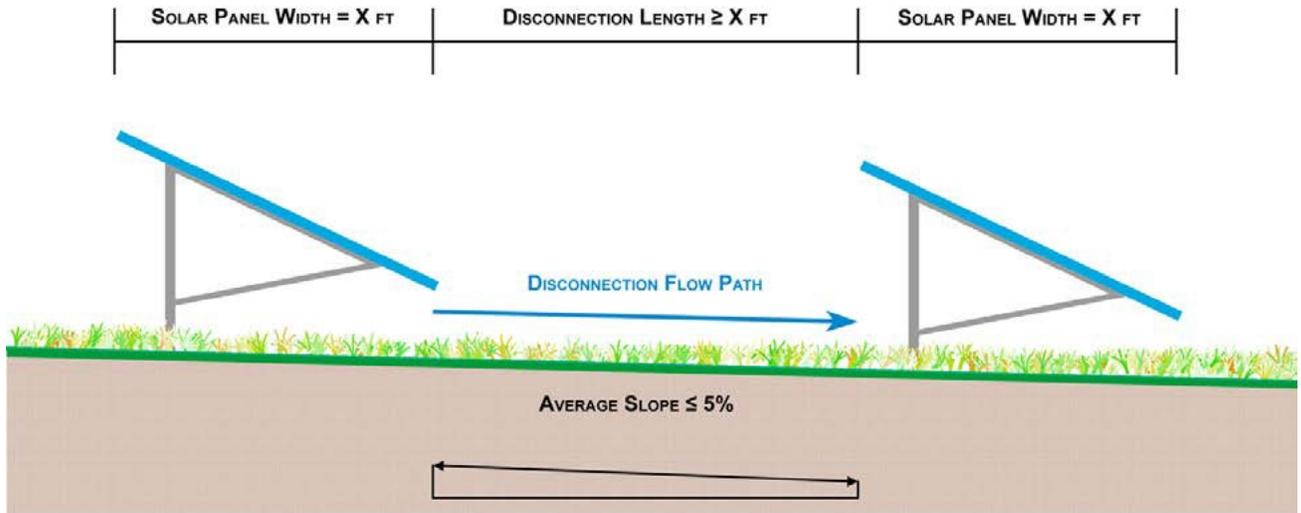
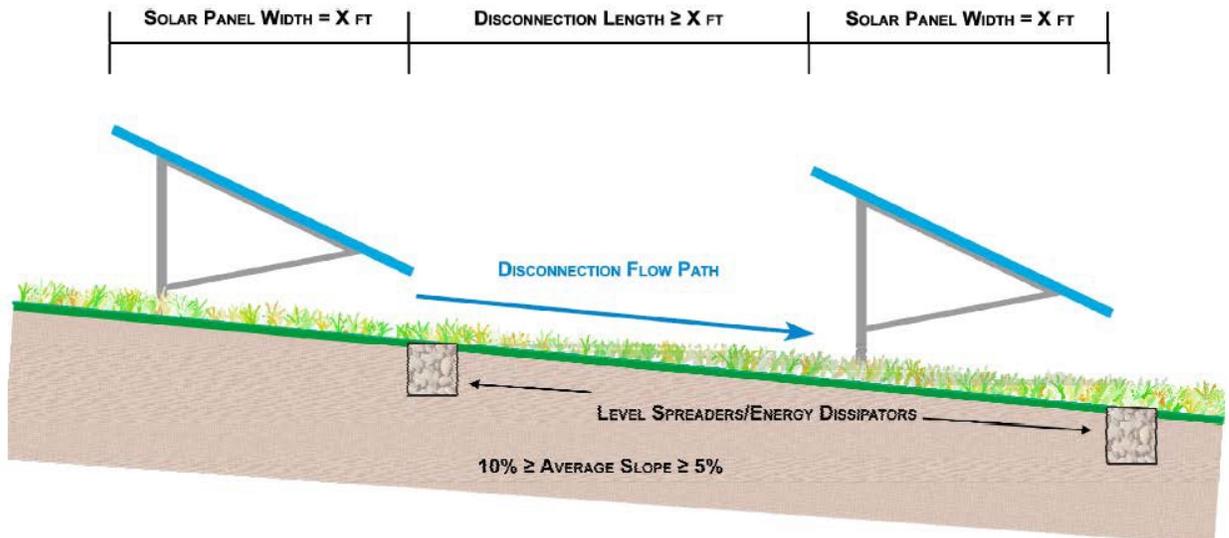


Figure 10-7
Typical Solar Panel Installation with Slopes $> 5\%$ and $\leq 10\%$



Source: Maryland Department of the Environment: Stormwater Design Guidance – Solar Panel Installations

APPENDIX A

ABBREVIATIONS AND DEFINITIONS



APPENDIX A

ABBREVIATIONS AND DEFINITIONS

ABBREVIATIONS

BFE	Base Flood Elevation
BMP	Best Management Practice
CFS	Cubic Feet Per Second
CLOMR	Conditional Letter of Map Revision (from FEMA)
CLOMR-F	Conditional Letter of Map Revision Based on Fill (from FEMA)
CN	Curve Number
COE	United States Army Corps of Engineers
CSMP	Comprehensive Stormwater Management Program
CSO	Combined Sewer Overflow
CWA	Clean Water Act
ERM	Elevation Reference Mark
E&SC	Erosion and Sediment Control
EPA	Environmental Protection Agency
ETJ	Extraterritorial Jurisdiction
FBFM	Flood Boundary and Floodway Map
FEH	Fluvial Erosion Hazard
FEMA	Federal Emergency Management Agency
FHBM	Flood Hazard Boundary Map
FIRM	Flood Insurance Rate Map
FIS	Flood Insurance Study
FPG	Flood Protection Grade
FPS	Feet Per Second
GIS	Geographical Information System
GPS	Global Positioning System
HGL	Hydraulic Grade Line

HHW	Household Hazardous Waste
HUC	Hydrologic Unit Code
IDEM	Indiana Department of Environmental Management
IDNR	Indiana Department of Natural Resources
INDOT	Indiana Department of Transportation.
LAG	Lowest Adjacent Grade
LOMA	Letter of Map Amendment (from FEMA)
LOMR	Letter of Map Revision (from FEMA)
LOMR-F	Letter of Map Revision Based on Fill (from FEMA)
MCM	Minimum Control Measure
MS4	Municipal Separate Storm Sewer System
NAVD	North American Vertical Datum of 1988
NFIP	National Flood Insurance Program
NGVD 1929	National Geodetic Vertical Datum of 1929
NRCS	USDA-Natural Resources Conservation Service
NPDES	National Pollution Discharge Elimination System
NPS	Non-point source
POTW	Publicly Owned Treatment Works
SFHA	Special Flood Hazard Area
SWCD	Soil and Water Conservation District
SWPPP	Stormwater Pollution Prevention Plan
SWQMP	Stormwater Quality Management Plan
Tc	Time of Concentration
TMDL	Total Maximum Daily Load
USCS	Unified Soil Classification System
USDA	United States Department of Agriculture
USFWS	United States Fish and Wildlife Service

DEFINITIONS

Acre-Foot (AF). A measure of water volume equal to the inundation of a flat one-acre area to a depth of one foot (43,560 cubic feet).

Administering authority. The designated unit of government given the authority to issue permits.

Agricultural land disturbing activity. Tillage, planting, cultivation, or harvesting operations for the production of agricultural or nursery vegetative crops. The term also includes pasture renovation and establishment, the construction of agricultural conservation practices, and the installation and maintenance of agricultural drainage tile. For purposes of this Technical Standards, the term does not include land disturbing activities for the construction of agricultural related facilities, such as barns, buildings to house livestock, roads associated with infrastructure, agricultural waste lagoons and facilities, lakes and ponds, wetlands; and other infrastructure.

Agricultural land use conservation practices. Use of land for the production of animal or plant life, including forestry, pasturing or yarding of livestock, and planting, growing, cultivating, and harvesting crops for human or livestock consumption. Practices that are constructed on agricultural land for the purposes of controlling soil erosion and sedimentation. These practices include grass waterways, sediment basins, terraces, and grade stabilization structures.

Amortization Period. The length of time used to repay a debt or mortgage or to depreciate an initial cost.

Antecedent Runoff Condition. The index of runoff potential before a storm event. The index, developed by the Soil Conservation Service (SCS), is an attempt to account for the variation of the SCS runoff curve number (CN) from storm to storm.

Backflow Preventer. Device that allows liquids to flow in only one direction in a pipe. Backflow preventers are used on sewer pipes to prevent a reverse flow during flooding situations.

Backwater. The rise in water surface elevation caused by some obstruction such as a narrow bridge opening, buildings or fill material that limits the area through which the water shall flow.

Base Flood Elevation. The water surface elevation corresponding to a flood having a one percent probability of being equaled or exceeded in a given year.

Base Flood. See "Regulatory Flood".

Base Flow. Stream discharge derived from groundwater sources as differentiated from surface runoff. Sometimes considered to include flows from regulated lakes or reservoirs.

Basement. A building story that is all or partly underground but having at least one-half of its height below the average level of the adjoining ground. A basement shall not be counted as a story for the purpose of height regulations.

Benchmark. A marked point of known elevation from which other elevations may be established.

Best Management Practices. Design, construction, and maintenance practices and criteria for stormwater facilities that minimize the impact of stormwater runoff rates and volumes, prevent erosion, and capture pollutants.

Buffer Strip. An existing, variable width strip of vegetated land intended to protect water quality and habitat.

Building. See “structure”.

Capacity of a Storm Drainage Facility. The maximum flow that can be conveyed or stored by a storm drainage facility without causing damage to public or private property.

Catch Basin. A chamber usually built at the curb line of a street for the admission of surface water to a storm drain or subdrain, having at its base a sediment sump designed to retain grit and detritus below the point of overflow.

Centerline of Channel. The thalweg of a channel.

Channel Improvement. Alteration, maintenance, or reconstruction of the channel area for the purpose of improving the channel capacity or overall drainage efficiency. The noted “improvement” does not necessarily imply water quality or habitat improvement within the channel or its adjacent area.

Channel Modification. Alteration of a channel by changing the physical dimensions or materials of its bed or banks. Channel modification includes damming, rip-rapping or other armoring, widening, deepening, straightening, relocating, lining, and significant removal of bottom or woody vegetation. Channel modification does not include the clearing of dead or dying vegetation, debris, or trash from the channel. Channelization is a severe form of channel modification typically involving relocation of the existing channel (e.g., straightening).

Channel Stabilization. Protecting the sides and bed of a channel from erosion by controlling flow velocities and flow directions using jetties, drops, or other structures and/or by fining the channel with vegetation, riprap, concrete, or other suitable lining material.

Channel. A portion of a natural or artificial watercourse which periodically or continuously contains moving water, or which forms a connecting link between two bodies of water. It has a defined bed and banks which serve to confine the water.

Class V injection well. A type of well, which typically has a depth greater than its largest surface dimension, emplaces fluids into the subsurface, and does not meet the definitions of Class I through Class IV wells as defined under 40 CFR 146.5. While the term includes the specific examples described in 40 CFR 144.81, septic systems that serve more than one (1) single-family dwelling or provide service for non-domestic waste, dug wells, bored wells, improved sinkholes, french drains, infiltration sumps, and infiltration galleries, it does not include surface impoundments, trenches, or ditches that are wider than they are deep.

Closed Conduit. A pipe, tube, or tile used for transmitting water.

Combined Sewer Overflow. A system designed and used to receive and transport combined sewage so that during dry periods the wastewater is carried to a treatment facility. During storm events, the excess water is discharged directly into a river, stream, or lake without treatment.

Compensatory Storage. An artificial volume of storage within a floodplain used to balance the loss of natural flood storage capacity when artificial fill or substructures are placed within the floodplain.

Compost. Organic residue (or a mixture of organic residue and soil) that has undergone biological decomposition until it has become relatively stable humus.

Comprehensive Stormwater Management Program. A comprehensive stormwater program for effective management of stormwater quantity and quality throughout the community.

Constructed Wetland. A manmade shallow pool that creates growing conditions suitable for wetland vegetation and is designed to maximize pollutant removal.

Construction activity. Land disturbing activities, and land disturbing activities associated with the construction of infrastructure and structures. This term does not include routine ditch or road maintenance or minor landscaping projects.

Construction plan. A representation of a project site and all activities associated with the project. The plan includes the location of the project site, buildings and other infrastructure, grading activities, schedules for implementation and other pertinent information related to the project site. A storm water pollution prevention plan is a part of the construction plan.

Construction site access. A stabilized stone surface at all points of ingress or egress to a project site, for the purpose of capturing and detaining sediment carried by tires of vehicles or other equipment entering or exiting the project site.

Contiguous. Adjoining or in actual contact with.

Contour Line. Line on a map which represents a contour or points of equal elevation.

Contour. An imaginary line on the surface of the earth connecting points of the same elevation.

Contractor or subcontractor. An individual or company hired by the project site or individual lot owner, their agent, or the individual lot operator to perform services on the project site.

Control Structure. A structure designed to control the rate of flow that passes through the structure, given a specific upstream and downstream water surface elevation.

Conveyance. Any structural method for transferring stormwater between at least two points. The term includes piping, ditches, swales, curbs, gutters, catch basins, channels, storm drains, and roadways.

Convolution. The process of translating precipitation excess into a runoff hydrograph.

Crawl Space. Low space below first floor of a house where there has not been excavation deep enough for a basement, usually less than seven (7) feet in depth, but where there is access for pipes, ducts, utilities and similar equipment.

Critical Duration Analysis. The process of testing different rainfall durations to find that “critical duration”, which produces the highest peak runoff or the highest storage volume.

Cross-Section. A graph or plot of ground elevation across a stream valley or a portion of it, usually along a line perpendicular to the stream or direction of flow.

Crown of Pipe. The elevation of top of pipe.

Cubic Feet Per Second (CFS). Used to describe the amount of flow passing a given point in a stream channel. One cubic foot per second is equivalent to approximately 7.5 gallons per second.

Culvert. A closed conduit used for the conveyance of surface drainage water under a roadway, railroad, canal or other impediment.

Curve Number (CN). The Soil Conservation Service index that represents the combined hydrologic effect of soil, land use, land cover, hydrologic condition and antecedent runoff condition.

Dam. A barrier to confine or impound water for storage or diversion, to prevent gully erosion, or to retain soil, sediment, or other debris.

Damage. Measurable rise in flood heights on buildings currently subject to flooding, flooding of buildings currently not subject to flooding and increases in volume or velocity to the point where the rate of land lost to erosion and scour is substantially increased.

Datum. Any level surface to which elevations are referred, usually Mean Sea Level.

Dechlorinated swimming pool discharge. Chlorinated water that has either sat idle for seven (7) days following chlorination prior to discharge to the MS4 conveyance, or, by analysis, does not contain detectable concentrations (less than five-hundredths (0.05) milligram per liter) of chlorinated residual.

Depressional Storage Areas. Non-riverine depressions in the earth where stormwater collects. The volumes are often referred to in units of acre-feet.

Design Storm. A selected storm event, described in terms of the probability of occurring once within a given number of years, for which drainage or flood control improvements are designed and built.

Detention Basin. A facility constructed or modified to restrict the flow of storm water to a prescribed maximum rate, and to detain concurrently the excess waters that accumulate behind the outlet.

Detention Facility. A facility designed to detain a specified amount of stormwater runoff assuming a specified release rate. The volumes are often referred to in units of acre-feet.

Detention Storage. The temporary detaining or storage of stormwater in storage facilities, on rooftops, in streets, parking lots, school yards, parks, open spaces or other areas under predetermined and controlled conditions, with the rate of release regulated by appropriately installed devices.

Detention Time. The theoretical time required to displace the contents of a tank or unit at a given rate of discharge (volume divided by rate of discharge).

Detention. Managing stormwater runoff by temporary holding and controlled release.

Detritus. Dead or decaying organic matter; generally contributed to stormwater as fallen leaves and sticks or as dead aquatic organisms.

Developer. Any person financially responsible for construction activity, or an owner of property who sells or leases, or offers for sale or lease, any lots in a subdivision.

Development. Any man-made change to improved or unimproved real estate including but not limited to:

- Construction, reconstruction, or placement of a building or any addition to a building;
- Construction of flood control structures such as levees, dikes, dams or channel improvements;
- Construction or reconstruction of bridges or culverts;
- Installing a manufactured home on a site, preparing a site for a manufactured home, or installing a recreational vehicle on a site for more than hundred eight (180) days;
- Installing utilities, erection of walls, construction of roads, or similar projects;
- Mining, dredging, filling, grading, excavation, or drilling operations;

- Storage of materials; or
- Any other activity that might change the direction, height, or velocity of flood or surface waters.

“Development” does not include activities such as the maintenance of existing buildings and facilities such as painting, re-roofing, resurfacing roads, or gardening, plowing and similar agricultural practices that do not involve filling, grading, excavation, or the construction of permanent buildings.

Direct Release. A method of stormwater management where runoff from a part or the entire development is released directly to the receiving stream without providing detention.

Discharge. In the context of water quantity provisions, usually the rate of water flow, i.e., a volume of fluid passing a point per unit time commonly expressed as cubic feet per second, cubic meters per second, gallons per minute, or millions of gallons per day. In the context of water quality provisions, the discharge means any addition of liquids or solids to a water body or a flow conveyance facility.

Disposal. The discharge, deposit, injection, spilling, leaking, or placing of any solid waste or hazardous waste into or on any land or water so that the solid waste or hazardous waste, or any constituent of the waste, may enter the environment, be emitted into the air, or be discharged into any waters, including groundwater.

Ditch. A man-made, open drainageway in or into which excess surface water or groundwater drained from land, stormwater runoff, or floodwaters flow either continuously or intermittently.

Drain. A buried slotted or perforated pipe or other conduit (subsurface drain) or a ditch (open drain) for carrying off surplus groundwater or surface water.

Drainage Area. The area draining into a stream at a given point. It may be of different sizes for surface runoff, subsurface flow and base flow, but generally the surface runoff area is considered as the drainage area.

Drainage Classification (soil). As a natural condition of the soil, drainage refers to both the frequency and duration of periods when the soil is free of saturation. Soil drainage conditions are defined as:

- *Well-drained*--Excess water drains away rapidly, and no mottling occurs within 36 in. of the surface.
- *Moderately well drained*--Water is removed from the soil somewhat slowly resulting in small but significant periods of wetness, and mottling occurs between 18 and 36 in.
- *Poorly drained*--Water is removed so slowly that it is wet for a large part of the time, and mottling occurs between 0 and 8 in.
- *Somewhat poorly drained*--Water is removed from the soil slowly enough to keep it wet for significant periods but not all of the time, and mottling occurs between 8 to 18 in.
- *Very poorly drained*--Water is removed so slowly that the water table remains at or near the surface for the greater part of the time; there may also be periods of surface ponding; the soil has a black to gray surface layer with mottles up to the surface.

Drainage. The removal of excess surface water or groundwater from land by means of ditches or subsurface drains. Also see Natural drainage.

Drop Manhole. Manhole having a vertical drop pipe connecting the inlet pipe to the outlet pipe. The vertical drop pipe shall be located immediately outside the manhole.

Dry Well. A type of infiltration practice that allows stormwater runoff to flow directly into the ground via a bored or otherwise excavated opening in the ground surface.

Dry-Bottom Detention Basin. A basin designed to be completely dewatered after having provided its planned detention of runoff during a storm event.

Duration. The time period of a rainfall event.

Earth Embankment. A man-made deposit of soil, rock, or other material often used to form an impoundment.

Elevation Certificate. A form published by the Federal Emergency Management Agency that is used to certify the 100-year or base flood elevation and the lowest elevation of usable space to which a building has been constructed.

Elevation Reference Mark (ERM). Elevation benchmark tied to the National Geodetic Vertical Datum of 1929 and identified during the preparation of a Flood Insurance Study prepared for the Federal Emergency Management Agency.

Emergency Spillway. Usually a vegetated earth channel used to safely convey flood discharges around an impoundment structure.

Energy Dissipater. A device to reduce the energy of flowing water.

Environment. The sum total of all the external conditions that may act upon a living organism or community to influence its development or existence.

Erosion and sediment control measure. A practice, or a combination of practices, to control erosion and resulting sedimentation. and/or off-site damages.

Erosion and sediment control system. The use of appropriate erosion and sediment control measures to minimize sedimentation by first reducing or eliminating erosion at the source and then as necessary, trapping sediment to prevent it from being discharged from or within a project site.

Erosion control plan. A written description and site plan of pertinent information concerning erosion control measures designed to meet the requirements of this Ordinance.

Erosion. The wearing away of the land surface by water, wind, ice, gravity, or other geological agents. The following terms are used to describe different types of water erosion:

- *Accelerated erosion*--Erosion much more rapid than normal or geologic erosion, primarily as a result of the activities of man.
- *Channel erosion* --An erosion process whereby the volume and velocity of flow wears away the bed and/or banks of a well-defined channel.
- *Gully erosion* --An erosion process whereby runoff water accumulates in narrow channels and, over relatively short periods, removes the soil to considerable depths, ranging from 1-2 ft. to as much as 75-100 ft.
- *Rill erosion*--An erosion process in which numerous small channels only several inches deep are formed; occurs mainly on recently disturbed and exposed soils (see Rill).

- *Splash erosion*--The spattering of small soil particles caused by the impact of raindrops on wet soils; the loosened and spattered particles may or may not be subsequently removed by surface runoff.
- *Sheet erosion*--The gradual removal of a fairly uniform layer of soil from the land surface by runoff water.

Extraterritorial Jurisdiction (ETJ). Areas located outside the corporate limits of a community over which the community has statutory development authority.

Farm or Field Tile. A pipe installed in an agricultural area to allow subsurface drainage of farmland for the purpose of agricultural production.

FEMA. The Federal Emergency Management Agency.

Filter Strip. Usually a long, relatively narrow area (usually, 20-75 feet wide) of undisturbed or planted vegetation used near disturbed or impervious surfaces to filter stormwater pollutants for the protection of watercourses, reservoirs, or adjacent properties.

Final stabilization. The establishment of permanent vegetative cover or the application of a permanent non-erosive material to areas where all land disturbing activities have been completed and no additional land disturbing activities are planned under the current permit.

Floatable. Any solid waste that will float on the surface of the water.

Flood (or Flood Waters). A general and temporary condition of partial or complete inundation of normally dry land areas from the overflow, the unusual and rapid accumulation, or the runoff of surface waters from any source.

Flood Boundary and Floodway Map (FBFM). A map prepared by the Federal Emergency Management Agency that depicts the FEMA designated floodways within a community. This map also includes delineation of the 100-year and 500-year floodplain boundaries and the location of the Flood Insurance Study cross-sections.

Flood Crest. The maximum stage or elevation reached or expected to be reached by the waters of a specific flood at a given time.

Flood Duration. The length of time a stream is above flood stage or overflowing its banks.

Flood Easement. Easement granted to identify areas inundated by the 100-year flood and prohibit or severely restrict development activities.

Flood Elevation. The elevation at all locations delineating the maximum level of high waters for a flood of given return period.

Flood Fighting. Actions taken immediately before or during a flood to protect human life and to reduce flood damages such as evacuation, emergency sandbagging and diking.

Flood Forecasting. The process of predicting the occurrence, magnitude and duration of an imminent flood through meteorological and hydrological observations and analysis.

Flood Frequency. A statistical expression of the average time period between floods equaling or exceeding a given magnitude. For example, a 100-year flood has a magnitude expected to be equaled or exceeded on the average of once every hundred years; such a flood has a one-percent chance of being equaled or exceeded in any given year. Often used interchangeably with "recurrence interval".

Flood Hazard Area. Any floodplain, floodway, floodway fringe, or any combination thereof which is subject to inundation by the regulatory flood; or any flood plain as delineated by Zone X on a Flood Hazard Boundary Map.

Flood Hazard Boundary Map (FHBM). A map prepared by the Federal Emergency Management Agency that depicts Special Flood Hazard Areas as a Zone A within a community. There are no study text, base flood elevations, or floodways associated with this map.

Flood Insurance Rate Map (FIRM). A map prepared by the Federal Emergency Management Agency that depicts Special Flood Hazard Areas within a community. This map also includes the 100-year or Base Flood Elevation at various locations along the watercourses. More recent versions of the FIRM may also show the FEMA designated floodway boundaries and the location of the Flood Insurance Study cross-sections.

Flood Insurance Study (FIS). A study prepared by the Federal Emergency Management Agency to assist a community participating in the National Flood Insurance Program in its application of the program regulations. The study consists of a text which contains community background information with respect to flooding, a floodway data table, summary of flood discharges, flood profiles, a Flood Insurance Rate Map, and a Flood Boundary and Floodway Map.

Flood Profile. A graph showing the relationship of water surface elevation to a specific location, the latter generally expressed as distance above the mouth of a stream of water flowing in a channel. It is generally drawn to show surface elevation for the crest or a specific magnitude of flooding, but may be prepared for conditions at any given time or stage.

Flood Protection Grade (FPG). The elevation of the regulatory or 100-year flood plus two (2) feet at any given location in the Special Flood Hazard Area or 100-year floodplain and/or adjacent regulated areas, if applicable.

Flood Resistant Construction (Flood Proofing). Additions, changes or adjustments to structures or property that are designed to reduce or eliminate the potential for flood damage.

Flood Storage Areas. Depressions, basins, or other areas that normally stand empty or partially empty, but fill with rainfall runoff during storms to hold the runoff and reduce downstream flow rates. The volumes are often referred to in units or acre-feet.

Floodplain Management. The operation of a program of corrective and preventive measures for reducing flood damage, including but not limited to flood control projects, floodplain land use regulations, flood proofing of buildings, and emergency preparedness plans.

Floodplain Regulations. General term applied to the full range of codes, ordinances and other regulations relating to the use of land and construction within floodplain limits. The term encompasses zoning ordinances, subdivision regulations, building and housing codes, encroachment laws and open area (space) regulations.

Floodplain. The channel proper and the areas adjoining the channel which have been or hereafter may be covered by the regulatory or 100-year flood. Any normally dry land area that is susceptible to being inundated by water from any natural source. The floodplain includes both the floodway and the floodway fringe districts.

Floodway Fringe. That portion of the flood plain lying outside the floodway, which is inundated by the regulatory flood.

Floodway. The channel of a river or stream and those portions of the floodplains adjoining the channel which are reasonably required to efficiently carry and discharge the peak flow of the regulatory flood of any river or stream.

Fluvial Erosion Hazard (FEH) Corridor. Fluvial (riverine) Erosion Hazard corridors represent the areas along the streams (including the channel and immediate overbanks areas) that are believed to be subject to stream movement or streambank erosion.

Footing Drain. A drain pipe installed around the exterior of a basement wall foundation to relieve water pressure caused by high groundwater elevation.

Freeboard. An increment of height added to the base flood elevation to provide a factor of safety for uncertainties in calculations, unknown local conditions, wave actions and unpredictable effects such as those caused by ice or debris jams. (See Flood Protection Grade).

French Drain. A drainage trench backfilled with a coarse, water-transmitting material; may contain a perforated pipe.

Gabion. An erosion control structure consisting of a wire cage or cages filled with rocks.

Garbage. All putrescible animal solid, vegetable solid, and semisolid wastes resulting from the processing, handling, preparation, cooking, serving, or consumption of food or food materials.

Geographical Information System. A computer system capable of assembling, storing, manipulation, and displaying geographically referenced information. This technology can be used for resource management and development planning.

Geotextile Fabric. A woven or non-woven, water-permeable synthetic material used to trap sediment particles, prevent the clogging of aggregates with fine grained soil particles, or as a separator under road aggregate.

Geotextile Liner. A synthetic, impermeable fabric used to seal impoundments against leaks.

Global Positioning System. A system that provides specially coded satellite signals that is processed by a receiver, which determines position, velocity, and time. The system is funded and controlled by the U.S. Department of Defense.

Grade. (1) The inclination or slope of a channel, canal, conduit, etc., or natural ground surface usually expressed in terms of the percentage the vertical rise (or fall) bears to the corresponding horizontal distance. (2) The finished surface of a canal bed, roadbed, top of embankment, or bottom of excavation; any surface prepared to a design elevation for the support of construction, such as paving or the laying of a conduit. (3) To finish the surface of a canal bed, roadbed, top of embankment, or bottom of excavation, or other land area to a smooth, even condition.

Grading. The cutting and filling of the land surface to a desired slope or elevation.

Grass. A member of the botanical family Graminae, characterized by blade-like leaves that originate as a sheath wrapped around the stem.

Grassed swale. A type of vegetative practice used to filter stormwater runoff via a vegetated, shallow-channel conveyance.

Grassed Waterway. A natural or constructed waterway, usually broad and shallow, covered with erosion-resistant grasses and used to conduct surface water from an area.

Ground Cover (horticulture). Low-growing, spreading plants useful for low-maintenance landscape areas.

Groundwater Recharge. The infiltration of water into the earth. It may increase the total amount of water stored underground or only replenish supplies depleted through pumping or natural discharge.

Groundwater. Accumulation of underground water, natural or artificial. The term does not include manmade underground storage or conveyance structures.

Habitat. The environment in which the life needs of a plant or animal are supplied.

Hard Surface. See “Impervious Surface.”

High Water. Maximum designed permitted or regulated water level for an impoundment.

Household Hazardous Waste. Solid waste generated by households that is ignitable, toxic, reactive, corrosive, or otherwise poses a threat to human health or the environment.

Hydraulic Grade Line (HGL). For Channel flow, the HGL is equal to the water surface whereas for pressure flow it is the piezometric surface.

Hydraulics. A branch of science that deals with the practical application of the mechanics of water movement. A typical hydraulic study is undertaken to calculate water surface elevations.

Hydrodynamic Loads. Forces imposed on structures by floodwaters due to the impact of moving water on the upstream side of the structure, drag along its sides, and eddies or negative pressures on its downstream side.

Hydrograph. For a given point on a stream, drainage basin, or a lake, a graph showing either the discharge, stage (depth), velocity, or volume of water with respect to time.

Hydrologic Unit Code. A numeric United States Geologic Survey code that corresponds to a watershed area. Each area also has a text description associated with the numeric code.

Hydrology. The science of the behavior of water in the atmosphere, on the surface of the earth, and underground. A typical hydrologic study is undertaken to compute flow rates associated with specified flood events.

Hydrometeorologic. Water-related meteorologic data such as rainfall or runoff.

Hydrostatic Loads. Those loads or pressures resulting from the static mass of water at any point of floodwater contact with a structure. They are equal in all direction and always act perpendicular to the surface on which they are applied. Hydrostatic loads can act vertically on structural members such as floors, decks and roofs, and can act laterally on upright structural members such as walls, piers, and foundations.

IDNR. Indiana Department of Natural Resources.

Illicit Discharge. Any discharge to a conveyance that is not composed entirely of stormwater except naturally occurring floatables, such as leaves or tree limbs.

Impact Areas. Areas defined or mapped that are unlikely to be easily drained because of one or more factors including but not limited to any of the following: soil type, topography, land where there is not adequate outlet, a floodway or floodplain, land within 75 feet of each bank of any regulated drain or within 75 feet from the centerline of any regulated tile ditch.

Impaired Waters. Waters that do not or are not expected to meet applicable water quality standards, as included on IDEM’s CWA Section 303(d) List of Impaired Waters.

Impervious surface. Surfaces, such as pavement and rooftops, which prevent the infiltration of stormwater into the soil.

Individual building lot. A single parcel of land within a multi-parcel development.

Individual lot operator. A contractor or subcontractor working on an individual lot.

Individual lot owner. A person who has financial control of construction activities for an individual lot.

INDOT. Indiana Department of Transportation. Generally used here to refer to specifications contained in the publication "INDOT Standard Specifications."

Infiltration practices. Any structural BMP designed to facilitate the percolation of run-off through the soil to ground water. Examples include infiltration basins or trenches, dry wells, and porous pavement.

Infiltration. Passage or movement of water into the soil.

Infiltration Swales. A depressed earthen area that is designed to promote infiltration.

Inlet. An opening into a storm drain system for the entrance of surface storm water runoff, more completely described as a storm drain inlet.

Intermittent Stream. An intermittent stream has flowing water during certain times of the year, when groundwater provides water for stream flow. During dry periods, intermittent streams may not have flowing water. Runoff from rainfall is a supplemental source of water for stream flow.

Invert. The inside bottom of a culvert or other conduit.

Junction Chamber. A converging section of conduit, usually large enough for a person to enter, used to facilitate the flow from one or more conduits into a main conduit.

Land Surveyor. A person licensed under the laws of the State of Indiana to practice land surveying.

Land-disturbing Activity. Any man-made change of the land surface, including removing vegetative cover that exposes the underlying soil, excavating, filling, transporting and grading.

Larger common plan of development or sale. A plan, undertaken by a single project site owner or a group of project site owners acting in concert, to offer lots for sale or lease; where such land is contiguous, or is known, designated, purchased or advertised as a common unit or by a common name, such land shall be presumed as being offered for sale or lease as part of a larger common plan. The term also includes phased or other construction activity by a single entity for its own use.

Lateral Storm Sewer. A drain that has inlets connected to it but has no other storm drain connected.

Life Cycle Cost. Cost based on the total cost incurred over the system life including research, development, testing, production, construction, operation, and maintenance. Costs are normally determined on present worth or equivalent annual cost basis.

Lowest Adjacent Grade. The elevation of the lowest grade adjacent to a structure, where the soil meets the foundation around the outside of the structure (including structural members such as basement walkout, patios, decks, porches, support posts or piers, and rim of the window well).

Major Drainage System. Drainage system carrying runoff from an area of one or more square miles.

Manhole. Storm drain structure through which a person may enter to gain access to an underground storm drain or enclosed structure.

Manning Roughness Coefficient or Manning's "n" Value. A dimensionless coefficient ("n") used in the Manning's equation to account for channel wall frictional losses in steady uniform flow.

Measurable storm event. A precipitation event that results in a total measured precipitation accumulation equal to, or greater than, one-half (0.5) inch of rainfall.

Minimum Control Measure. Minimum measures required by the NPDES Phase II program. The six (6) MCMs are: Public education and outreach, Public participation and involvement, Illicit discharge detection and elimination, Construction site runoff control, Post-construction runoff control, and Pollution prevention and good housekeeping.

Minor Drainage Systems. Drainage system carrying runoff from an area of less than one square mile.

Minor Subdivision. See Subdivision, Minor.

Mulch. A natural or artificial layer of plant residue or other materials covering the land surface which conserves moisture, holds soil in place, aids in establishing plant cover, and minimizes temperature fluctuations.

Multi-Family. Any structure which contains three or more dwelling units. A dwelling unit is any structure, or part of a structure, which is constructed to a house a family.

Municipal Separate Storm Sewers. An MS4 who meets all the following criteria: (1) is a conveyance or system of conveyances owned by the state, county, city, town, or other public entity; (2) discharges to waters of the U.S.; (3) is designed or used for collecting or conveying stormwater; (4) is not a combined sewer; and, (5) is not part of a Publicly Owned Treatment Works (POTW).

Municipal, state, federal, or institutional refueling area. An operating gasoline or diesel fueling area whose primary function is to provide fuel to either municipal, state, federal, or institutional equipment or vehicles.

Mutual Drain. A drain that: (1) Is located on two or more tracts of land that are under different ownership; (2) was established by the mutual consent of all the owners; and (3) was not established under or made subject to any drainage statute.

National Flood Insurance Program (NFIP). The NFIP is a Federal program enabling property owners to purchase flood insurance. The Federal Emergency Management Agency administers the NFIP in communities throughout the United States. The NFIP is based on an agreement between local communities and the Federal government which states that if a community will implement floodplain management measures to reduce future flood risks to new construction and substantially improved structures in flood hazard areas, the Federal government will make flood insurance available within the community as a financial protection against flood losses that do occur.

National Geodetic Vertical Datum of 1929. The nationwide, Federal Elevation datum used to reference topographic elevations to a known value.

National Pollution Discharge Elimination System (NPDES). A permit developed by the U.S. EPA through the Clean Water Act. In Indiana, the permitting process has been delegated to IDEM. This permit covers aspects of municipal stormwater quality.

Natural Drainage. The flow patterns of stormwater run-off over the land in its pre-development state.

Nonagricultural land use. Commercial use of land for the manufacturing and wholesale or retail sale of goods or services, residential or institutional use of land intended primarily to shelter people, highway use of land including lanes, alleys, and streets, and other land uses not included in agricultural land use.

Nonpoint Source Pollution. Pollution that enters a water body from diffuse origins on the watershed and does not result from discernable, confined, or discrete conveyances.

Normal Depth. Depth of flow in an open conduit during uniform flow for the given conditions.

North American Vertical Datum of 1988 (NAVD 1988). The nationwide, federal elevation datum used to reference topographic elevations to a known value.

Nutrient(s). (1) A substance necessary for the growth and reproduction of organisms. (2) In water, those substances (chiefly nitrates and phosphates) that promote growth of algae and bacteria.

Off-site. Everything not located at or within a particular site.

Off-site Land Areas. Those areas that by virtue of existing topography naturally shed surface water onto or through the developing property.

100-Year Frequency Flood. See “regulatory flood”.

On-Site. Located within the controlled or urbanized area where runoff originates.

Open Drain. A natural watercourse or constructed open channel that conveys drainage water.

Open Space. Any land area devoid of any disturbed or impervious surfaces created by industrial, commercial, residential, agricultural, or other manmade activities.

Orifice. A device which controls the rate of flow from a detention basin.

Outfall scouring. The deterioration of a streambed or lakebed from an outfall discharge to an extent that the excessive settling of solid material results and suitable aquatic habitat is diminished.

Outfall. The point, location, or structure where a pipe or open drain discharges to a receiving body of water.

Outlet. The point of water disposal from a stream, river, lake, tidewater, or artificial drain.

Overland Flow. Consists of sheet flow, shallow concentrated flow and channel flow.

Peak Discharge (or Peak Flow). The maximum instantaneous flow from a given storm condition at a specific location.

Percolation. The movement of water through soil.

Perennial Stream. A stream that maintains water in its channel throughout the year.

Permanent stabilization. The establishment, at a uniform density of seventy percent (70%) across the disturbed area, of vegetative cover or permanent non-erosive material that will ensure the resistance of the soil to erosion, sliding, or other movement.

Permeability (soil). The quality of a soil that enables water or air to move through it. Usually expressed in inches per hour or inches per day.

Pervious. Allowing movement of water.

Pesticides. Chemical compounds used for the control of undesirable plants, animals, or insects. The term includes insecticides, herbicides, algicides, rodenticides, nematocides, fungicides, and growth regulators.

pH. A numerical measure of hydrogen ion activity, the neutral point being 7.0. All pH values below 7.0 are acid, and all above 7.0 are alkaline.

Phasing of construction. Sequential development of smaller portions of a large project site, stabilizing each portion before beginning land disturbance on subsequent portions, to minimize exposure of disturbed land to erosion.

Phosphorus (available). Inorganic phosphorus that is readily available for plant growth.

Piping. The formation of "pipes" by underground erosion. Water in the soil carries the fine soil particles away, and a series of eroded tubes or tunnels develop. These openings will grow progressively larger and can cause a dam failure.

Planimetric Data. Horizontal measurements involving distances or dimensions on a diagram, map, Plat of Survey or topographic map. Normally in units of feet.

Plat of Survey. A scaled diagram showing boundaries of a tract of land or subdivision. This may constitute a legal description of the land and be used in lieu of a written description.

Point Source. Any discernible, confined, and discrete conveyance including but not limited to any pipe, ditch, channel, tunnel, conduit, well, discrete fissure, or container from which pollutants are or maybe discharged (P.L. 92-500, Section 502[14]).

Pollutant of concern. Any pollutant that has been documented via analytical data as a cause of impairment in any waterbody.

Porosity. The volume of pore space in soil or rock.

Porous pavement. A type of infiltration practice to improve the quality and reduce the quantity of storm water run-off via the use of manmade, pervious pavement which allows run-off to percolate through the pavement and into underlying soils

Private Drain. A drain that: (1) Is located on land owned by one person or by two or more persons jointly; and (2) was not established under or made subject to any drainage statute.

Professional Engineer. A person licensed under the laws of the State of Indiana to practice professional engineering.

Programmatic Indicator. Any data collected by an MS4 entity that is used to indicate implementation of one (1) or more minimum control measures.

Project site owner. The person required to submit a stormwater permit application, and required to comply with the terms of this ordinance, including a developer or a person who has financial and operational control of construction activities, and project plans and specifications, including the ability to make modifications to those plans and specifications.

Project site. The entire area on which construction activity is to be performed.

Probable Maximum Flood. The most severe flood that may be expected from a combination of the most critical meteorological and hydrological conditions that are reasonably possible in the drainage basin. It is used in designing high-risk flood protection works and siting of structures and facilities that shall be subject to almost no risk of flooding. The probable maximum flood is usually much larger than the 100-year flood.

Publically Owned Treatment Works (POTW). A municipal operation that breaks down and removes contaminants in the wastewater prior to discharging to a stream through primary and/or secondary treatment systems.

Qualified professional. An individual who is trained and experienced in storm water treatment techniques and related fields as may be demonstrated by state registration, professional certification, experience, or completion of coursework that enable the individual to make sound, professional judgments regarding storm water control or treatment and monitoring, pollutant fate and transport, and drainage planning.

Radius of Curvature. Length of radius of a circle used to define a curve.

Rain garden. A vegetative practice used to alter impervious surfaces, such as roofs, into pervious surfaces for absorption and treatment of rainfall.

Rainfall Intensity. The rate at which rain is falling at any given instant, usually expressed in inches per hour.

Reach. Any length of river, channel or storm drain.

Receiving Stream or Receiving Water. The body of water into which runoff or effluent is discharged. The term does not include private drains, unnamed conveyances, retention and detention basins, or constructed wetlands used as treatment.

Recharge. Replenishment of groundwater reservoirs by infiltration and transmission from the outcrop of an aquifer or from permeable soils.

Recurrence Interval. A statistical expression of the average time between floods equaling or exceeding a given magnitude.

Redevelopment. Alterations of a property that change a site or building in such a way that there is disturbances of one (1) acre or more of land. The term does not include such activities as exterior remodeling.

Regional Pond. A detention/retention basin sized to detain/retain the runoff from the entire watershed, on-site and off-site, tributary to the pond's outlet.

Regulated Drain. A drain subject to the provisions of the Indiana Drainage Code, I.C.-36-9-27.

Regulatory or 100-Year Flood. The discharge or elevation associated with the 100-year flood as calculated by a method and procedure which is acceptable to and approved by the Indiana Department of Natural Resources and the Federal Emergency Management Agency. The "regulatory flood" is also known as the "base flood".

Regulatory Floodway. See Floodway.

Release Rate - The amount of storm water release from a storm water control facility per unit of time.

Reservoir. A natural or artificially created pond, lake or other space used for storage, regulation or control of water. May be either permanent or temporary. The term is also used in the hydrologic modeling of storage facilities.

Residential Land. Used for housing or dwelling units.

Retail gasoline outlet. An operating gasoline or diesel fueling facility whose primary function is the resale of fuels. The term applies to facilities that create five thousand (5,000) or more square feet of impervious surfaces, or generate an average daily traffic count of one hundred (100) vehicles per one thousand (1,000) square feet of land area.

Retention basin. A type of storage practice, that has no positive outlet, used to retain storm water run-off for an indefinite amount of time. Runoff from this type of basin is removed only by infiltration through a porous bottom or by evaporation.

Retention. The storage of stormwater to prevent it from leaving the development site. May be temporary or permanent.

Retention Facility. A facility designed to completely retain a specified amount of stormwater runoff without release except by means of evaporation, infiltration or pumping. The volumes are often referred to in units of acre-feet.

Return Period - The average interval of time within which a given rainfall event will be equaled or exceeded once. A flood having a return period of 100 years has a one percent probability of being equaled or exceeded in any one year.

Revetment. Facing of stone or other material, either permanent or temporary, placed along the edge of a stream to stabilize the bank and protect it from the erosive action of the stream. Also see Revetment riprap.

Right-of-Way for a County Drain. The statutory right of way as defined by Indiana Code for a regulated drain.

Riparian habitat. A land area adjacent to a waterbody that supports animal and plant life associated with that waterbody.

Riparian zone. Of, on, or pertaining to the banks of a stream, river, or pond.

Riprap. Broken rock, cobble, or boulders placed on earth surfaces, such as the face of a dam or the bank of a stream, for protection against the action of water (waves). Revetment riprap is material graded such that: (1) no individual piece weighs more than 120 lbs. and (2) 90-100% will pass through a 12-inch sieve, 20-60% through a 6-inch sieve, and not more than 10% through a 12-inch sieve.

River Restoration. Restoring the channel of a stream or ditch to its perceived original, non-obstructed capacity by means of clearing & snagging, obstruction removal, and inexpensive streambank protection measures. The term "restoration", as noted, does not necessarily imply restoration or improvement of water quality or habitat within the channel or its adjacent area.

Riverine. Relating to, formed by, or resembling a stream (including creeks and rivers).

Runoff Coefficient - A decimal fraction relating the amount of rain which appears as runoff and reaches the storm drain system to the total amount of rain falling. A coefficient of 0.5 implies that 50 percent of the rain falling on a given surface appears as storm water runoff.

Runoff. That portion of precipitation that flows from a drainage area on the land surface, in open channels, or in stormwater conveyance systems.

Sand. (1) Soil particles between 0.05 and 2.0 mm in diameter. (2) A soil textural class inclusive of all soils that are at least 70% sand and 15% or less clay.

Sanitary Backup. The condition where a sanitary sewer reaches capacity and surcharges into the lowest area.

Scour. The clearing and digging action of flowing water.

Sediment. Solid material (both mineral and organic) that is in suspension, is being transported, or has been moved from its site of origin by air, water, gravity, or ice and has come to rest on the earth's surface.

Sedimentation. The process that deposits soils, debris and other unconsolidated materials either on the ground surfaces or in bodies of water or watercourses.

Seepage. The passage of water or other fluid through a porous medium, such as the passage of water through an earth embankment or masonry wall.

Sensitive Water. A water body in need of priority protection or remediation based on its:

providing habitat for threatened or endangered species,

usage as a public water supply intake,

relevant community value,

usage for full body contact recreation,

exceptional use classification as found in 327 IAC 2-1-11(b), outstanding state resource water classification as found in 327 IAC 2-1-2(3) and 327 IAC 2-1.5-19(b).

Settling Basin. An enlargement in the channel of a stream to permit the settling of debris carried in suspension.

Silt Fence. A fence constructed of wood or steel supports and either natural (e.g. burlap) or synthetic fabric stretched across area of non-concentrated flow during site development to trap and retain on-site sediment due to rainfall runoff.

Silt. (1) Soil fraction consisting of particles between 0.002 and 0.05 mm in diameter. (2) A soil textural class indicating more than 80% silt.

Siphon - A closed conduit or portion of which lies above the hydraulic grade line, resulting in a pressure less than atmospheric and requiring a vacuum within the conduit to start flow. A siphon utilizes atmospheric pressure to effect or increase the flow of water through a conduit. An inverted siphon is used to carry storm water flow under an obstruction such as a sanitary sewer.

Site. The entire area included in the legal description of the land on which land disturbing activity is to be performed.

Slope. Degree of deviation of a surface from the horizontal, measured as a numerical ratio or percent. Expressed as a ratio, the first number is commonly the horizontal distance (run) and the second is the vertical distance (rise)--e.g., 2:1. However, the preferred method for designation of slopes is to clearly identify the horizontal (H) and vertical (V) components (length (L) and Width (W) components for horizontal angles). Also note that according to international standards (Metric), the slopes are presented as the vertical or width component shown on the numerator--e.g., 1V:2H. Slope expressions in this ordinance follow the common presentation of slopes--e.g., 2:1 with the metric presentation shown in parenthesis--e.g., (1V:2H). Slopes can also be

expressed in "percents". Slopes given in percents are always expressed as (100*V/H) --e.g., a 2:1 (1V:2H) slope is a 50% slope.

Soil and Water Conservation District. A public organization created under state law as a special-purpose district to develop and carry out a program of soil, water, and related resource conservation, use, and development within its boundaries. A subdivision of state government with a local governing body, established under IC 14-32.

Soil. The unconsolidated mineral and organic material on the immediate surface of the earth that serves as a natural medium for the growth of land plants.

Solid Waste. Any garbage, refuse, debris, or other discarded material.

Special Flood Hazard Area. An area that is inundated during the 100-Year flood.

Spill. The unexpected, unintended, abnormal, or unapproved dumping, leakage, drainage, seepage, discharge, or other loss of petroleum, hazardous substances, extremely hazardous substances, or objectionable substances. The term does not include releases to impervious surfaces when the substance does not migrate off the surface or penetrate the surface and enter the soil.

Spillway - A waterway in or about a hydraulic structure, for the escape of excess water.

Standard Project Flood. A term used by the U.S. Army Corps of Engineers to designate a flood that may be expected from the most severe combination of meteorological and hydrological conditions that are considered reasonable characteristics of the geographical area in which the drainage basin is located, excluding extremely rare combinations. The peak flow for a standard project flood is generally 40 – 60 percent of the probable maximum flood for the same location.

Stilling Basin - A basin used to slow water down or dissipate its energy.

Storage practices. Any structural BMP intended to store or detain stormwater and slowly release it to receiving waters or drainage systems. The term includes detention and retention basins.

Storm drain signing. Any marking procedure that identifies a storm sewer inlet as draining directly to a receiving waterbody so as to avoid dumping pollutants. The procedures can include painted or cast messages and adhesive decals.

Storm Duration. The length of time that water may be stored in any stormwater control facility, computed from the time water first begins to be stored.

Storm Event. An estimate of the expected amount of precipitation within a given period of time. For example, a 10-yr. frequency, 24-hr. duration storm event is a storm that has a 10% probability of occurring in any one year. Precipitation is measured over a 24-hr. period.

Storm Frequency. The time interval between major storms of predetermined intensity and volumes of runoff--e.g., a 5-yr., 10-yr. or 20-yr. storm.

Storm Sewer. A closed conduit for conveying collected storm water, while excluding sewage and industrial wastes. Also called a storm drain.

Stormwater Drainage System - All means, natural or man-made, used for conducting storm water to, through or from a drainage area to any of the following: conduits and appurtenant features, canals, channels, ditches, storage facilities, swales, streams, culverts, streets and pumping stations.

Stormwater Facility. All ditches, channels, conduits, levees, ponds, natural and manmade impoundments, wetlands, tiles, swales, sewers and other natural or artificial means of draining surface and subsurface water from land.

Stormwater Pollution Prevention Plan. A plan developed to minimize the impact of storm water pollutants resulting from construction activities.

Stormwater Quality Management Plan. A comprehensive written document that addresses stormwater runoff quality.

Stormwater Quality Measure. A practice, or a combination of practices, to control or minimize pollutants associated with storm water runoff.

Stormwater runoff. The water derived from rains falling within a tributary basin, flowing over the surface of the ground or collected in channels or conduits.

Stormwater. Water resulting from rain, melting or melted snow, hail, or sleet.

Stream Gauging. The quantitative determination of streamflow using gauges, current meters, weirs, or other measuring instruments at selected locations (see Gauging station').

Stream Length. The length of a stream or ditch, expressed in miles, from the confluence of the stream or ditch with the receiving stream to the upstream extremity of the stream or ditch, as indicated by the solid or dashed, blue or purple line depicting the stream or ditch on the most current edition of the seven and one-half (72) minute topographic quadrangle map published by the United States Geological Survey, measured along the meanders of the stream or ditch as depicted on the map.

Stream. See Intermittent stream, Perennial stream, Receiving stream.

Streambanks. The usual boundaries (not the flood boundaries) of a stream channel. Right and left banks are named facing downstream.

Strip development. A multi-lot project where building lots front on an existing road.

Structure. Refers to a structure that is principally above ground and is enclosed by walls and a roof. The term includes but is not limited to, a gas or liquid storage tank, a manufactured home or a prefabricated building, and recreational vehicles to be installed on a site for more than 180 days.

Structural Engineer. A person licensed under the laws of the State of Indiana to engage in the designing or supervising of construction, enlargement or alteration of structures or any part thereof.

Structural Floodplain. Management Measures. Those physical or engineering measures employed to modify the way floods behave, (e.g., dams, dikes, levees, channel enlargements and diversions).

Subarea/Subbasin. Portion of a watershed divided into homogenous drainage units which can be modeled for purposes of determining runoff rates. The subareas/subbasins have distinct boundaries, as defined by the topography of the area.

Subdivision.

1. The division of a parent tract or other piece of land into at least two smaller lots so that either now or in the future the subdivider can do any of the following with one or more of the subdivided lots:
 - a. Transfer ownership;
 - b. Construct buildings;

- c. Create new building sites for leasehold.
2. The actual location, shape and size of a parent tract to be divided is determined by the official record of the last transfer of its ownership transacted before this chapter was enacted or by its last conditional transfer of ownership by recorded contract transacted before this chapter was enacted. The following kinds of divisions are not subdivisions and are exempt from the rules of this chapter:
 - a. A division of land into two or more tracts all of which are at least ten acres in size;
 - b. A division of land for the sale or exchange of tracts to correct errors in an existing legal description; provided, that no additional building sites other than for accessory buildings are created by the division;
 - c. A division of land pursuant to an allocation of land in the settlement of a decedent's estate or a court decree for the distribution of property;
 - d. A division of land for the acquisition of street right-of-way, or easement; and
 - e. A division of land for the sale or exchange of tracts between adjoining land owners, provided that no additional building sites other than for accessory buildings are created by the division.
3. Additionally, any division of land complying fully with Section 14.02.050 is not a subdivision and shall be called a parcelization.

Subdivision, Minor. Any subdivision of not more than four lots, all with frontage on a perimeter street, or any further subdivision or re-subdivision of a nonresidential development whose lots may or may not have frontage on a perimeter street which does not include any improvement to a public road; provided, such subdivision activity involves neither the construction of any new street or road, nor the imposition of any adverse effect upon the use of the remainder of the land or adjoining property as determined by the Commission; and provided, such activity conflicts with neither any provision or portion of the Comprehensive Plan, official map, or unified zoning code, unified subdivision ordinance or any of the regulations set forth herein.

Subsoil. The B horizons of soils with distinct profiles. In soils with weak profile development, the subsoil can be defined as the soil below which roots do not normally grow.

Subsurface Drain. A pervious backfield trench, usually containing stone and perforated pipe, for intercepting groundwater or seepage.

Subwatershed. A watershed subdivision of unspecified size that forms a convenient natural unit. See also Subarea.

Sump Failure. A failure of the sump pump that results in inundation of crawl space or basement.

Sump Pump. A pump that discharges seepage from foundation footing drains.

Surcharge. Backup of water in a sanitary or storm sewer system in excess of the design capacity of the system.

Surface Runoff. Precipitation that flows onto the surfaces of roofs, streets, the ground, etc., and is not absorbed or retained by that surface but collects and runs off.

Suspended Solids. Solids either floating or suspended in water.

Swale. An elongated depression in the land surface that is at least seasonally wet, is usually heavily vegetated, and is normally without flowing water. Swales conduct stormwater into primary drainage channels and may provide some groundwater recharge.

Tailwater. The water surface elevation at the downstream side of a hydraulic structure (i.e. culvert, bridge, weir, dam, etc.).

Temporary Stabilization. The covering of soil to ensure its resistance to erosion, sliding, or other movement. The term includes vegetative cover, anchored mulch, or other non-erosive material applied at a uniform density of seventy percent (70%) across the disturbed area.

Thalweg. The deepest point (or centerline) of a channel.

Tile Drain. Pipe made of perforated plastic, burned clay, concrete, or similar material, laid to a designed grade and depth, to collect and carry excess water from the soil.

Tile Drainage. Land drainage by means of a series of tile lines laid at a specified depth, grade, and spacing.

Time of Concentration (tc). The travel time of a particle of water from the most hydraulically remote point in the contributing area to the point under study. This can be considered the sum of an overland flow time and times of travel in street gutters, storm sewers, drainage channels, and all other drainage ways.

Topographic Map. Graphical portrayal of the topographic features of a land area, showing both the horizontal distances between the features and their elevations above a given datum.

Topography. The representation of a portion of the earth's surface showing natural and man-made features of a give locality such as rivers, streams, ditches, lakes, roads, buildings and most importantly, variations in ground elevations for the terrain of the area.

Topsoil. (1) The dark-colored surface layer, or a horizon, of a soil; when present it ranges in depth from a fraction of an inch to 2-3 ft. (2) Equivalent to the plow layer of cultivated soils. (3) Commonly used to refer to the surface layer(s), enriched in organic matter and having textural and structural characteristics favorable for plant growth.

Total Maximum Daily Load. Method used to establish allowable loadings for specified pollutants in a surface water resource to meet established water quality standards.

Toxicity. The characteristic of being poisonous or harmful to plant or animal life. The relative degree or severity of this characteristic.

TP-40 Rainfall. Design storm rainfall depth data for various durations published by the National Weather Service in their Technical Paper 40 dated 1961.

Trained individual. An individual who is trained and experienced in the principles of storm water quality, including erosion and sediment control as may be demonstrated by state registration, professional certification (such as CESSWI and/or CPESC certification), or other documented and applicable experience, or coursework as deemed sufficient by the City of Lafayette that enable the individual to make judgments regarding storm water control or treatment and monitoring.

Transition Section. Reaches of the stream of floodway where water flows from a narrow cross-section to a wide cross-section or vice-versa.

Tributary. Based on the size of the contributing drainage area, a smaller watercourse which flows into a larger watercourse.

Turbidity. (1) Cloudiness of a liquid, caused by suspended solids. (2) A measure of the suspended solids in a liquid.

Underdrain. A small diameter perforated pipe that allows the bottom of a detention basin, channel or swale to drain.

Unified Soil Classification System. A system of classifying soils that is based on their identification according to particle size, gradation, plasticity index, and liquid limit.

Uniform Flow. A state of steady flow when the mean velocity and cross-sectional area remain constant in all sections of a reach.

Unit Hydrograph. A unit hydrograph is the hydrograph that results from one inch of precipitation excess generated uniformly over the watershed at a uniform rate during a specified period of time.

Urban Drain. A drain defined as “Urban Drain” in Indiana Drainage Code.

Urbanization The development, change or improvement of any parcel of land consisting of one or more lots for residential, commercial, industrial, institutional, recreational or public utility purposes.

Vegetative practices. Any nonstructural or structural BMP that, with optimal design and good soil conditions, utilizes various forms of vegetation to enhance pollutant removal, maintain and improve natural site hydrology, promote healthier habitats, and increase aesthetic appeal. Examples include grass swales, filter strips, buffer strips, constructed wetlands, and rain gardens.

Vegetative Stabilization. Protection of erodible or sediment producing areas with: permanent seeding (producing long-term vegetative cover), short-term seeding (producing temporary vegetative cover), or sodding (producing areas covered with a turf of perennial sod-forming grass).

Water Course. Any river, stream, creek, brook, branch, natural or man-made drainage way in or into which stormwater runoff or floodwaters flow either regularly or intermittently.

Water Quality. A term used to describe the chemical, physical, and biological characteristics of water, usually in respect to its suitability for a particular purpose.

Water Resources. The supply of groundwater and surface water in a given area.

Water Table. (1) The free surface of the groundwater. (2) That surface subject to atmospheric pressure under the ground, generally rising and falling with the season or from other conditions such as water withdrawal.

Waterbody. Any accumulation of water, surface, or underground, natural or artificial.

Watercourse. Any river, stream, creek, brook, branch, natural or man-made drainageway in or into which stormwater runoff or floodwaters flow either continuously or intermittently.

Watershed Area. All land and water within the confines of a drainage divide. See also Watershed.

Watershed. The region drained by or contributing water to a specific point that could be along a stream, lake or other stormwater facilities. Watersheds are often broken down into subareas for the purpose of hydrologic modeling.

Waterway. A naturally existing or manmade open conduit or channel utilized for the conveyance of water.

Weir. A channel-spanning structure for measuring or regulating the flow of water.

Wellhead protection area. Has the meaning set forth at 327 IAC 8-4.1-1(27).

Wet-Bottom Detention Basin (Retention Basin) - A basin designed to retain a permanent pool of water after having provided its planned detention of runoff during a storm event.

Wetlands. Areas that are inundated or saturated by surface water or groundwater at a frequency and duration sufficient to support, and that under normal circumstances do support, a prevalence of vegetation typically adapted for life in saturated soil conditions.

APPENDIX B

APPLICATIONS, FORMS AND MISCELLANEOUS SHEETS REQUIRED TO COMPLY WITH THE CITY OF LAFAYETTE STORMWATER ORDINANCE AND TECHNICAL STANDARDS

B1 –Stormwater Permit Forms

Application/Checklist for Stormwater Permit
Construction/ SWPPP Technical Review Form
BMP Maintenance Agreement

B2 – Construction Inspection/Completion Forms

Construction Self-Monitoring Report
Certification of Completion & Compliance
Termination Inspection Checklist
Certified As-Built Plans Checklist

B3 – Individual Lot/Parcel Permit Forms

Individual Lot Stormwater Pollution Prevention Requirements
Individual Lot Certification Form

B1 –Stormwater Permit Forms

Application/Checklist for Stormwater Permit
Construction/ SWPPP Technical Review Form
BMP Maintenance Agreement

City of Lafayette
Application/Checklist for Stormwater Permit
 (To Be Completed by Applicant)

Project Name:	
General Location:	
Form Completed By (Name):	Date Completed:
Total Site Acreage:	Proposed Land Disturbance Acreage:

1. Application Fee

Check Attached	Amt. \$
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2. Owner/Applicant Information

Owner Name:	Phone #:	E-Mail:
Engineer Company Name:		
Engineer Name:	Phone #:	E-Mail:

Brief Project Purpose and Description:

3. Construction Plans – General Requirements	Page/Sheet #
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3.1	Title sheet which includes location map, vicinity map, operating authority, design company name, developer name, and index of plan sheets.	
3.2	A copy of a legal boundary survey for the site, performed in accordance with Rule 12 of Title 865 of the Indiana Administrative Code or any applicable and subsequently adopted rule or regulation for the subdivision limits, including all drainage easements and wetlands.	
3.3	A reduced plat or project site map showing the parcel identification numbers, the lot numbers, lot boundaries, easements, and road layout and names. The reduced map must be legible and submitted on a sheet or sheets no larger than eleven (11) inches by seventeen (17) inches for all phases or sections of the project site.	
3.4	An existing project site layout that must include the following information:	
3.4a	A topographic map of the land to be developed and such adjoining land whose topography may affect the layout or drainage of the development. The contour intervals shall be one (1) foot when slopes are less than or equal to two percent (<2%) and shall be two (2) feet when slopes exceed two percent (>2%). All elevations shall be given in either National Geodetic Vertical Datum of 1929 (NGVD) or North American Vertical Datum of 1988 (NAVD). The horizontal datum of topographic map shall be based on Indiana State Plane Coordinates, NAD83. The map will contain a notation indicating these datum information. <ul style="list-style-type: none"> i. If the project site is less than or equal to two (2) acres in total land area, the topographic map shall include all topography of land surrounding the site to a distance of at least one hundred (100) feet. ii. If the project site is greater than two (2) acres in total land area, the topographic map shall include all topography of land surrounding the site to a distance of at least two hundred (200) feet. 	
3.4b	Location, name, and normal water level of all wetlands, lakes, ponds, and water courses on or adjacent to the project site.	
3.4c	Location of storm, sanitary, combined sewer, and septic tank systems and outfalls.	
3.4d	The location of regulated drains, farm drains, inlets and outfalls, if any of record.	
3.4e	Location of all existing cornerstones within the proposed development and a plan to protect and preserve them.	

3.5	A grading and drainage plan, including the following information:		
3.5a	Location of all proposed site improvements, including roads, utilities, lot delineation and identification, proposed structures, and common areas.		
3.5b	Delineation of all proposed land disturbing activities, including off-site activities that will provide services to the project site.		
3.5c	Information regarding any off-site borrow, stockpile, or disposal areas that are associated with a project site, regardless of who owns or controls those areas. Off-site disposal areas may need to have their own permits.		
3.5d	Location, size, and dimensions of all existing streams to be maintained, and new drainage systems such as culverts, bridges, storm sewers, conveyance channels, and 100-year overflow paths/ponding areas shown as hatched areas, along with the associated easements.		
3.5e	Location, size, and dimensions of features such as permanent retention or detention facilities, including existing or manmade wetlands, used for the purpose of stormwater management. Include existing retention or detention facilities that will be maintained, enlarged, or otherwise altered and new ponds or basins to be built.		
3.5f	One or more typical cross sections of all existing and proposed channels or other open drainage facilities carried to a point above the 100-year high water and showing the elevation of the existing land and the proposed changes, together with the high water elevations expected from the 100 year storm under the controlled conditions called for by this ordinance, and the relationship of structures, streets, and other facilities.		
3.6	Utility plan sheet(s) showing the location of all proposed utility lines for the project		
3.7	Storm sewer plan/profile sheet(s) showing the elevation, size, length, location of all proposed storm sewers. Existing and proposed ground grades, storm sewer structures elevations, and utility crossings also must be included.		
3.8	A 24-inch by 36-inch plat, including the following information:		
3.8a	Legal description.		
3.8b	Cross reference to Rule 12.		
3.8c	Regulated drain statement and table.		
3.9	Any other information required by Tippecanoe County to thoroughly evaluate the submitted material.		
4. Stormwater Drainage Technical Report			Page/Sheet #
4.1	A summary report, including the following information:		
4.1a	Description of the nature and purpose of the project.		
4.1b	The significant drainage problems associated with the project.		
4.1c	The analysis procedure used to evaluate these problems and to propose solutions.		
4.1d	Any assumptions or special conditions associated with the use of these procedures, especially the hydrologic or hydraulic methods.		
4.1e	The proposed design of the drainage control system.		
4.1f	The results of the analysis of the proposed drainage control system showing that it does solve the project's drainage problems. Any hydrologic or hydraulic calculations or modeling results must be adequately cited and described in the summary description. If hydrologic or hydraulic models are used, the input and output files for all necessary runs must be included in the appendices. A map showing any drainage area subdivisions used in the analysis must accompany the report.		
4.1g	<p>A discussion of the anticipated earthwork balance for the project site (For purposes of this section of the Ordinance, an earthwork balance occurs if project site material export/import occurs within the overall masterplan footprint). Indicate if the project site is expected to:</p> <ul style="list-style-type: none"> • have an earthwork balance, or • if material export/import is expected outside of the overall masterplan footprint, include the expected amount of material export/import. <p>If material export/import outside of the overall masterplan footprint is anticipated, indicate the export/import location(s) and the status of local, state, and/or federal permits needed at these off-</p>		

		site locations, based on the information available at the time of project submittal. If earthwork export/import is expected outside of the overall masterplan footprint, a condition will be added to the project approval that the export/import location(s) and proof of proper permits for any export/import locations must be provided at the pre-construction meeting. If an earthwork balance is expected, no condition will be added to the project approval.	
	4.1h	Proof of Errors and Omissions Insurance for the registered professional engineer or licensed surveyor showing a minimum amount of \$1,000,000 in coverage.	
4.2	A Hydrologic/Hydraulic Analysis, consistent with the methodologies and calculation included in the technical standards, and including the following information:		
	4.2a	A hydraulic report detailing existing and proposed drainage patterns on the subject site. The report should include a description of present land use and proposed land use. Any off-site drainage entering the site should be addressed as well. This report should be comprehensive and detail all of the steps the engineer took during the design process.	
	4.2b	All hydrologic and hydraulic computations should be included in the submittal. These calculations should include, but are not limited to: runoff curve numbers and runoff coefficients, runoff calculations, stage-discharge relationships, times-of-concentration and storage volumes.	
	4.2c	Copies of all computer runs. These computer runs should include both the input and the outputs. Electronic copies of the computer runs with input files will expedite the review process and is required to be submitted.	
	4.2d	A set of exhibits should be included showing the drainage sub-areas and a schematic detailing of how the computer models were set up.	
	4.2e	A conclusion which summarizes the hydraulic design and details how this design satisfies this Ordinance.	
5. Stormwater Pollution Prevention Plan for Construction Sites			Page/Sheet #
5.1	Construction Plan Elements (Section A in IDEM Form)		
	A1	Index of the location of required plan elements in the construction plan	
	A2	A vicinity map depicting the project site location in relationship to recognizable local landmarks, towns, and major roads	
	A3	Narrative of the nature and purpose of the project	
	A4	Latitude and longitude to the nearest fifteen (15) seconds	
	A5	Legal description of the project site	
	A6	11 X 17-inch plat showing building lot numbers/boundaries and road layout/names	
	A7	Boundaries of the one hundred (100) year floodplains, floodway fringes, and floodways	
	A8	Land use of all adjacent properties	
	A9	Identification of a U.S. EPA approved or established TMDL	
	A10	Name(s) of the receiving water(s)	
	A11	Identification of discharges to a water on the current 303d list of impaired waters and the pollutant(s) for which it is impaired	
	A12	Soil map of the predominant soil types	
	A13	Identification and location of all known wetlands, lakes and water courses on or adjacent to the project site (construction plan, existing site layout)	
	A14	Identification of any other state or federal water quality permits or authorizations that are required for construction activities	

A15	Identification and delineation of existing cover, including natural buffers	
A16	Existing topography at a contour interval appropriate to indicate drainage patterns	
A17	Location(s) of where run-off enters the project site	
A18	Location(s) of where run-off discharges from the project site prior to land disturbance	
A19	Location of all existing structures on the project site	
A20	Existing permanent retention or detention facilities, including manmade wetlands, designed for the purpose of stormwater management	
A21	Locations where stormwater may be directly discharged into ground water, such as abandoned wells, sinkholes, or karst features	
A22	Size of the project area expressed in acres	
A23	Total expected land disturbance expressed in acres	
A24	Proposed final topography	
A25	Locations and approximate boundaries of all disturbed areas	
A26	Location, size, and dimensions of all stormwater drainage systems, such as culverts, storm sewers, and conveyance channels	
A27	Locations of specific points where stormwater and non-stormwater discharges will leave the project site	
A28	Location of all proposed site improvements, including roads, utilities, lot delineation and identification, proposed structures, and common areas	
A29	Location of all on-site soil stockpiles and borrow areas	
A30	Construction support activities that are expected to be part of the project	
A31	Location of any in-stream activities that are planned for the project including, but not limited to stream crossings and pump arounds	
5.2	Erosion and Sediment Control/Project Site Management (Section B in IDEM Form)	
B1	Description of the potential pollutant generating sources and pollutants, including all potential non-stormwater discharges	
Where applicable, Items in B2 through B10 below will be evaluated for location, dimensions, detailed specifications, and construction details		
B2	Stable construction entrance locations and specifications	
B3	Specifications for temporary and permanent stabilization	
B4	Sediment control measures for concentrated flow areas	
B5	Sediment control measures for sheet flow areas	
B6	Run-off control measures	
B7	Stormwater outlet protection locations and specifications	
B8	Grade stabilization structure locations and specifications	
B9	Dewatering applications and management methods	

	B10	Measures utilized for work within waterbodies	
	B11	Maintenance guidelines for each proposed temporary stormwater quality measure	
	B12	Planned construction sequence describing the relationship between implementation of stormwater quality measures in relation to land disturbance	
	B13	Provisions for erosion and sediment control on individual building lots regulated under the proposed project	
	B14	Material handling, spill prevention and spill response plan meeting requirements in 327 IAC 2-6.1	
	B15	Material handling and storage procedures associated with construction activity	
6. Post-Construction Stormwater Pollution Prevention Plan			Page/Sheet #
6.1	SWPPP – Post-Construction (Section C in IDEM Form)		
	C1	Description of pollutants and their sources associated with the proposed land use	
	C2	Description of proposed post-construction stormwater measures	
	C3	Plan details for each stormwater measure	
	C4	Sequence describing stormwater measure implementation	
	C5	Maintenance guidelines for proposed post-construction stormwater measures	
	C6	Entity that will be responsible for operation and maintenance of the post-construction stormwater measures	
7. Project Check-in Checklist			Page/Sheet #
Must be completed and all items listed must be provided at project submittal.			

<input type="checkbox"/>	Update and submit a document (narrative and/or plan sheets) that address plan deficiencies.
<input type="checkbox"/>	Update and submit a complete plan set that addresses plan deficiencies. A comprehensive plan review will not be completed.

Plan Review Information

- *The technical review and comment is intended to evaluate the completeness of the Construction/Stormwater Pollution Prevention Plan for the project. The Plan submitted was not reviewed for the adequacy of engineering design. All measures included in the plan, as well as those recommended in the comments should be evaluated as to their feasibility by a qualified individual with structural measures designed by a qualified engineer. The Plan has not been reviewed for other local, state, or federal permits that may be required to proceed with this project.*
- *Additional information, including design calculations may be requested to further evaluate the plan.*
- *All proposed stormwater pollution prevention measures and those referenced in this review must meet the design criteria and standards set forth in the "Indiana Stormwater Quality Manual" from the Indiana Department of Environmental Management or similar Guidance Documents.*
- *Construction activities and unforeseen weather conditions may affect the performance of the erosion and sediment control system, individual measures, or the effectiveness of the plan. The plan must be a flexible document, with provisions to modify or substitute measures as necessary to ensure compliance.*

Section A: Construction Plan Elements

Adequate	Deficient	NA	A	The construction plan elements include general information associated with the project site that are critical for the evaluation of the stormwater pollution prevention plan component. This information includes, but is not limited to an index, resource information, reference maps, grading information, project layout and design, and drainage plan
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	1	Index of the location of required plan elements in the construction plan
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	2	A vicinity map depicting the project site location in relationship to recognizable local landmarks, towns, and major roads
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	3	Narrative of the nature and purpose of the project
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	4	Latitude and longitude to the nearest fifteen (15) seconds
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	5	Legal description of the project site
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	6	11 X 17-inch plat showing building lot numbers/boundaries and road layout/names
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	7	Boundaries of the one hundred (100) year floodplains, floodway fringes, and floodways
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	8	Land use of all adjacent properties
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	9	Identification of a U.S. EPA approved or established TMDL
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	10	Name(s) of the receiving water(s)
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	11	Identification of discharges to a water on the current 303d list of impaired waters and the pollutant(s) for which it is impaired
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	12	Soil map of the predominant soil types
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	13	Identification and location of all known wetlands, lakes and water courses on or adjacent to the project site (construction plan, existing site layout)
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	14	Identification of any other state or federal water quality permits or authorizations that are required for construction activities
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	15	Identification and delineation of existing cover, including natural buffers
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	16	Existing topography at a contour interval appropriate to indicate drainage patterns

<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	17	Location(s) of where run-off enters the project site
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	18	Location(s) of where run-off discharges from the project site prior to land disturbance
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	19	Location of all existing structures on the project site
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	20	Existing permanent retention or detention facilities, including manmade wetlands, designed for the purpose of stormwater management
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	21	Locations where stormwater may be directly discharged into ground water, such as abandoned wells, sinkholes, or karst features
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	22	Size of the project area expressed in acres
Adequate	Deficient	NA	A	<i>The construction plan elements include general information associated with the project site that are critical for the evaluation of the stormwater pollution prevention plan component. This information includes, but is not limited to an index, resource information, reference maps, grading information, project layout and design, and drainage plan</i>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	23	Total expected land disturbance expressed in acres
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	24	Proposed final topography
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	25	Locations and approximate boundaries of all disturbed areas
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	26	Location, size, and dimensions of all stormwater drainage systems, such as culverts, storm sewers, and conveyance channels
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	27	Locations of specific points where stormwater and non-stormwater discharges will leave the project site
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	28	Location of all proposed site improvements, including roads, utilities, lot delineation and identification, proposed structures, and common areas
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	29	Location of all on-site soil stockpiles and borrow areas
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	30	Construction support activities that are expected to be part of the project
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	31	Location of any in-stream activities that are planned for the project including, but not limited to stream crossings and pump arounds

Section A – Comments:

- Evaluate areas with potential waters of the state and, where required, verify if permits/authorizations are required prior to any impacts to waters of the state. These potential resources include areas with hydric soil, hydrophytic vegetation, pooling water, or evidence of flowing water such as swales, ditches, drains, or natural conveyances. Evaluation of hydric soil, hydrophytic vegetation, or pooling water should conform to the US Army Corps of Engineers Wetlands Delineation Manual," Technical Report Y-87-1, and the applicable regional supplement https://www.usace.army.mil/Missions/Civil-Works/Regulatory-Program-and-Permits/reg_supp/. Avoidance and minimization of impacts to waters of the state should be prioritized.

Section B: Stormwater Pollution Prevention Plan – Erosion and Sediment Control/Project Site Management

Adequate	Deficient	NA	B	<i>The construction component of the Stormwater Pollution Prevention Plan includes stormwater quality measures to address erosion, sedimentation, and other pollutants associated with land disturbance and construction activities. Proper implementation of the plan, maintenance of measures, and administering a self-monitoring program is required to manage the project site to minimize the discharge of sediment and other pollutants. Construction activities and unforeseen weather conditions may affect the performance of the erosion and sediment control system, individual measures, or the effectiveness of the plan. The plan must be a flexible document, with provisions to modify or substitute measures as necessary to ensure compliance.</i>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	1	Description of the potential pollutant generating sources and pollutants, including all potential non-stormwater discharges
Where applicable, Items in 2 through 10 below will be evaluated for Location, dimensions, detailed specifications, and construction details				
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	2	Stable construction entrance locations and specifications
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	3	Specifications for temporary and permanent stabilization
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	4	Sediment control measures for concentrated flow areas
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	5	Sediment control measures for sheet flow areas
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	6	Run-off control measures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	7	Stormwater outlet protection locations and specifications
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	8	Grade stabilization structure locations and specifications
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	9	Dewatering applications and management methods
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	10	Measures utilized for work within waterbodies
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	11	Maintenance guidelines for each proposed temporary stormwater quality measure
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	12	Planned construction sequence describing the relationship between implementation of stormwater quality measures in relation to land disturbance
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	13	Provisions for erosion and sediment control on individual building lots regulated under the proposed project
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	14	Material handling and spill prevention and spill response plan meeting the requirements in 327 IAC 2-6.1
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	15	Material handling and storage procedures associated with construction activity

Section B – Comments:

- Stormwater quality measures for the reduction of sediment have not been evaluated for adequacy of design. The proposed measures included in this SWPPP are being accepted based on the design engineer’s submittal.

Section C: Stormwater Pollution Prevention Plan – Post-Construction

Adequate	Deficient	NA	C	
				<i>The post-construction component of the Stormwater Pollution Prevention Plan includes the implementation of stormwater quality measures to address pollutants that will be associated with the final project land use. Post-construction stormwater measures should be functional upon completion of the project. Long term functionality of the measures is critical to their performance and should be monitored and maintained.</i>
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	1	Description of pollutants and their sources associated with the proposed land use
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	2	Description of proposed post-construction stormwater measures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	3	Plan details for each stormwater measure
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	4	Sequence describing stormwater measure implementation
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	5	Maintenance guidelines for proposed post-construction stormwater measures
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	6	Entity that will be responsible for operation and maintenance of the post-construction stormwater measures

Section C – Comments:

- Post-construction stormwater quality and quantity measures have not been evaluated for adequacy of design. The proposed measures included in this SWPPP are being accepted based on the design engineer’s submittal.
- The rate of stormwater run-off and/or volume from the project site must meet local requirements to address stormwater quantity as established by ordinance or other regulatory mechanism. When a local requirement does not exist, the post-development run-off discharge from the project site must not exceed the pre-development discharge based on the two-year, ten-year, and one-hundred-year peak storm events.

Stormwater Management BMPs Maintenance Agreement

THIS AGREEMENT is made this _____ day of _____, 20____, by _____ [Owner Name] of _____ [Company Name] with principal offices located _____ [Owner/Company Address], hereinafter "Owner".

In accordance with Tippecanoe County Stormwater Ordinance and Technical Standards, the Owner agrees to install and maintain stormwater management practice(s) (also known as BMPs) on the subject property, known as _____ [Property's Common Name] located at _____ [Property's Address], hereinafter "Property" in accordance with Exhibit A. The Owner further agrees to the terms stated in this document to ensure that the stormwater management practice(s) continues serving the intended function in perpetuity. This Agreement includes the following exhibit:

Exhibit A: BMP Operation and Maintenance Manual ("Manual").

Note: This agreement and all Exhibits shall be recorded with the deed of property by the Owner at the Tippecanoe County Recorder's Office and two (2) copies of the recorded document provided to Tippecanoe County Surveyor's office, hereinafter "Community".

Through this Agreement, the Owner hereby subjects the Property to the following covenants, conditions, and restrictions:

- D.* The Owner shall be solely responsible for the installation, maintenance, and repair of the stormwater management practices, drainage easements, and associated landscaping identified in the Manual.
- E.* No alterations or changes to the stormwater management practice(s) identified in the Manual shall be permitted unless they are deemed to comply with this Agreement and are approved in writing by the Community.
- F.* The Owner shall retain the services of a qualified individual or company to operate and ensure the maintenance of the stormwater management practice(s) identified in the Manual.
- G.* The Owner shall annually, by December 30th, provide to the Community records of inspections, maintenance, and repair of the stormwater management practices in accordance with the Manual.
- H.* The Community or its designee is authorized to access the property as necessary to conduct inspections of the stormwater management practices or drainage easements to ascertain compliance with the intent of this Agreement and the activities prescribed in the Manual. Upon written notification by the Community or its designee of required maintenance or repairs, the Owner shall complete the specified maintenance or repairs within a reasonable time frame determined by the Community. The Owner(s) shall be liable for the failure to undertake any maintenance or repairs so that the public health, safety and welfare shall not be endangered nor the road improvement damaged.
- I.* If the Owner fails to properly maintain the stormwater management practice(s) in accordance with the Manual and this Agreement, the Community is authorized, but not required, to perform the specified inspections, maintenance, or repairs in order to preserve the intended functions of the practice(s) and prevent the practice(s) from becoming a threat to public health, safety, general welfare or the environment. In the case of an emergency, as determined by the Community, no notice shall be required prior to the Community performing emergency maintenance or repairs. The Community may levy the costs and expenses of such inspections, maintenance, or repairs plus a ten percent (10%) administrative fee against the Owner. The Community at the time of entering upon said stormwater management practice for the purpose of maintenance or repair may file a notice of lien in the office of the Register of Deeds of Tippecanoe County upon the property affected by the lien. If said costs and expenses are not paid by the Owner, the Community may pursue the collection of same through appropriate court actions and in

such a case, the Owner shall pay in addition to said costs and expenses all costs of litigation, including attorney fees.

- J.* The Owner hereby conveys to the Community an easement over, on, and in the Property or otherwise grants perpetual access rights for the purpose of access to the stormwater management practice for the inspection, maintenance, and repair thereof, should the Owner fail to properly inspect, maintain, and repair the practice(s).
- K.* The Owner agrees that this Agreement shall be recorded and that the Property shall be subject to the covenants and obligations contained herein, and this Agreement shall bind all current and future owners of the property.
- L.* The Owner agrees in the event that the Property is sold, transferred, or leased to provide information to the new owner, operator, or lessee regarding proper inspection, maintenance, and repair of the stormwater management practice(s). The information shall accompany the first deed transfer and include this Agreement and all Exhibits. The transfer of this information shall also be required with any subsequent sale, transfer, or lease of the Property.
- M.* The Owner agrees that the rights, obligations, and responsibilities hereunder shall commence upon execution of the Agreement.
- N.* The Owner whose signatures appear below hereby represent and warrant that they have the authority and capacity to sign this agreement and bind the respective parties hereto.
- O.* The Owner, its agents, representatives, successors, and assigns shall defend, indemnify and hold the Community harmless from and against any claims, demands, actions, damages, injuries, costs or expenses of any nature whatsoever, hereinafter "Claims", fixed or contingent, known or unknown, arising out of or in any way connected with the design, construction, use, maintenance, repair or operation (or omissions in such regard) of the stormwater management practice(s) referred to in Exhibit A which are the subject of this Agreement. This indemnity and hold harmless shall include any costs, expenses, and attorney fees incurred by the Community in connection with such Claims or the enforcement of this Agreement.

IN WITNESS WHEREOF, the Owner has executed this Agreement on the day and year first above written.

Owner Signature

Date

Printed Name

Company

Title

STATE OF INDIANA)
)
COUNTY OF TIPPECANOE)

SS:

BEFORE ME, the undersigned, a Notary Public in and for said County and State, personally appeared _____ Owner subscribed and sworn before this _____ day of _____, 20 ____.

Commission Expiration Date

County of Residence

Signature

Printed Name

WHEN RECORDED, RETURN FILE STAMPED COPY TO:

Tippecanoe County Drainage Board
20 N. Third Street
Lafayette, IN 47901

.....
Accepted by Tippecanoe County Drainage Board:

Signature: _____

Date: _____

Printed Name: _____

Title: _____

B2 – Construction Inspection/Completion Forms

Construction Self-Monitoring Report
Certification of Completion & Compliance
Termination Inspection Checklist
Certified As-Built Plans Checklist

Self-Monitoring Report (To Be Completed by Permittee's Trained Individual)

PART A: INSPECTION INFORMATION

Project Name: CSGP Project # Number: Permit # Assigned by MS4: Name of MS4: County: Name of Evaluator: Title of Evaluator: Affiliation: Email/Phone Number:	Inspection Type <input type="checkbox"/> 24 hr (after >.5" rain) <input type="checkbox"/> 24 hr (before >.5" rain) <input type="checkbox"/> Weekly <input type="checkbox"/> Monthly <i>(only applies to areas permanently stabilized)</i>	Inspection Date and Time <div style="text-align: center;">Click here to enter a date.</div> <hr/> Date of Last or Forecasted (circle one) Precipitation: <div style="text-align: center;">Click here to enter a date.</div> <hr/> Amount of Last or Forecasted (circle one) Precipitation: <div style="text-align: center;">Click here to enter a date.</div>
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PART B: SITE CONDITIONS

Current Site Information (check all that apply): Clearing Grubbing Grading Building Construction
 Installation of Infrastructure Utility Work Vegetative Establishment Other:

Soil Conditions: Dry Moist Wet Muddy Partial Snow Cover Snow Cover Frozen Freeze/Thaw
 Other:

PART C: PROJECT MANAGEMENT

(1) Was the SWP3 accessible at the time of the inspection?	<input type="checkbox"/> Yes <input type="checkbox"/> No
(2) Is the SWP3 current and/or updated to reflect the current stage of development?	<input type="checkbox"/> Yes <input type="checkbox"/> No
(3) Have all action items identified on the preceding reports been resolved? (a) If not identify which items require repair and provide a resolution timeline:	<input type="checkbox"/> Yes <input type="checkbox"/> No
(4) Is the project posting information posted in accordance with the CSGP Section 3.7?	<input type="checkbox"/> Yes <input type="checkbox"/> No

PART D: GUIDANCE FOR COMPLETION OF FORM

- Identify all areas of the site that currently have an erosion/sediment control measure in place. Using Part E, evaluate the current condition of the measure and using the middle column, determine what action needs to be taken.
- Identify all areas of the site where stormwater run-off leaves site or where any discharge occurs. In each location where run-off leave the site, evaluate:
 - If an erosion/sediment control measure is in place: evaluate each action step required in Part E
 - If no erosion/sediment control measure is in place or a measure is required, use Part E to include specifics about the location and type of measure to be implemented.
- For any area of the site where there is run-off, or a discharge provide a description and location using Part F.
 - If any sedimentation is occurring note where it is discharging: Off site, to a waterbody (on or off-site) or other sensitive area
- For Part H in the Evaluation of sheet flow and concentrated run-off:
 - Identify location of discharge/run-off and check any visual descriptions that apply to the discharge. If the discharge/run-off will resolve with the repair of a sediment or erosion control measure or good housekeeping practices, you do not need to list an action in the observations/notes section. If the discharge/run-off will not be solved with repair/replacement of a sediment control measure or good housekeeping practices, you will need to list the action taken to resolve the run-off and any additional pollutants visible in the discharge.
 - Print out additional pages for Parts E and F.

PART E: SEDIMENT CONTROL AND RUN-OFF MANAGEMENT

Measure: Location(s):	<input type="checkbox"/> No Action Required <input type="checkbox"/> Maintenance Required <input type="checkbox"/> Repair Measure <input type="checkbox"/> Temporary Measure <input type="checkbox"/> Replace Measure <input type="checkbox"/> Alternative Measure <input type="checkbox"/> Additional Measure	Observations/Notes:
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Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
--	---

Measure: Location(s):	<input type="checkbox"/> No Action Required <input type="checkbox"/> Maintenance Required <input type="checkbox"/> Repair Measure <input type="checkbox"/> Temporary Measure <input type="checkbox"/> Replace Measure <input type="checkbox"/> Alternative Measure <input type="checkbox"/> Additional Measure	Observations/Notes:
--	--	---------------------

Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
--	---

Measure: Location(s):	<input type="checkbox"/> No Action Required <input type="checkbox"/> Maintenance Required <input type="checkbox"/> Repair Measure <input type="checkbox"/> Temporary Measure <input type="checkbox"/> Replace Measure <input type="checkbox"/> Alternative Measure <input type="checkbox"/> Additional Measure	Observations/Notes:
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Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
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Measure: Location(s):	<input type="checkbox"/> No Action Required <input type="checkbox"/> Maintenance Required <input type="checkbox"/> Repair Measure <input type="checkbox"/> Temporary Measure <input type="checkbox"/> Replace Measure <input type="checkbox"/> Alternative Measure <input type="checkbox"/> Additional Measure	Observations/Notes:
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Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
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Measure: Location(s):	<input type="checkbox"/> No Action Required <input type="checkbox"/> Maintenance Required <input type="checkbox"/> Repair Measure <input type="checkbox"/> Temporary Measure <input type="checkbox"/> Replace Measure <input type="checkbox"/> Alternative Measure <input type="checkbox"/> Additional Measure	Observations/Notes:
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Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
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PART F: SURFACE STABILIZATION

Location(s):	<input type="checkbox"/> Permanent Vegetative Cover: <input type="checkbox"/> Continue to Monitor <input type="checkbox"/> 70 Percent Density Achieved <input type="checkbox"/> Perform Seeding/Reseed <input type="checkbox"/> Temporary <input type="checkbox"/> Permanent <input type="checkbox"/> Apply straw mulch and anchor <input type="checkbox"/> Install Erosion Control Blanket <input type="checkbox"/> Repair Erosion <input type="checkbox"/> Utilize Alternative Stabilization Method	Observations/Notes:
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Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
--	---

Location(s):	<input type="checkbox"/> Permanent Vegetative Cover: <input type="checkbox"/> Continue to Monitor <input type="checkbox"/> 70 Percent Density Achieved <input type="checkbox"/> Perform Seeding/Reseed <input type="checkbox"/> Temporary <input type="checkbox"/> Permanent <input type="checkbox"/> Apply straw mulch and anchor <input type="checkbox"/> Install Erosion Control Blanket <input type="checkbox"/> Repair Erosion <input type="checkbox"/> Utilize Alternative Stabilization Method	Observations/Notes:
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Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
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Location(s):	<input type="checkbox"/> Permanent Vegetative Cover: <input type="checkbox"/> Continue to Monitor <input type="checkbox"/> 70 Percent Density Achieved <input type="checkbox"/> Perform Seeding/Reseed <input type="checkbox"/> Temporary <input type="checkbox"/> Permanent <input type="checkbox"/> Apply straw mulch and anchor <input type="checkbox"/> Install Erosion Control Blanket <input type="checkbox"/> Repair Erosion <input type="checkbox"/> Utilize Alternative Stabilization Method	Observations/Notes:
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Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
--	---

Location(s):	<input type="checkbox"/> Permanent Vegetative Cover: <input type="checkbox"/> Continue to Monitor <input type="checkbox"/> 70 Percent Density Achieved <input type="checkbox"/> Perform Seeding/Reseed <input type="checkbox"/> Temporary <input type="checkbox"/> Permanent <input type="checkbox"/> Apply straw mulch and anchor <input type="checkbox"/> Install Erosion Control Blanket <input type="checkbox"/> Repair Erosion <input type="checkbox"/> Utilize Alternative Stabilization Method	Observations/Notes:
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Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
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PART G: GOOD HOUSEKEEPING

Site Ingress/Egress Location(s):	<input type="checkbox"/> Maintain Construction Entrance <input type="checkbox"/> Remove Tracked Sediment (do not flush sediment) <input type="checkbox"/> Install Additional Measures	Observations/Notes:
--	--	---------------------

Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
--	---

Trash Location(s):	<input type="checkbox"/> Continue to Monitor and Manage <input type="checkbox"/> Cover Trash Receptacles <input type="checkbox"/> Clean Up Wind-blown Trash <input type="checkbox"/> Other	Observations/Notes:
------------------------------	---	---------------------

Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
--	---

Spills or Leaks Location(s):	Type of Leak/Spill <input type="checkbox"/> Concrete and/or Cementitious Washout <input type="checkbox"/> Fuel <input type="checkbox"/> Other	Observations/Notes/Action Taken:
--	--	----------------------------------

Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
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PART H: EVALUATION OF SHEET FLOW AND CONCENTRATED RUN-OFF (DISCHARGES)

Location(s):	<input type="checkbox"/> Sediment <input type="checkbox"/> Oil Sheen <input type="checkbox"/> Odor <input type="checkbox"/> Floatables/Trash <input type="checkbox"/> Foam <input type="checkbox"/> Color/Turbid Discharge <input type="checkbox"/> Other	Observations/Notes:
--------------	---	---------------------

Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
--	---

Location(s):	<input type="checkbox"/> Sediment <input type="checkbox"/> Oil Sheen <input type="checkbox"/> Odor <input type="checkbox"/> Floatables/Trash <input type="checkbox"/> Foam <input type="checkbox"/> Color/Turbid Discharge <input type="checkbox"/> Other	Observations/Notes:
--------------	---	---------------------

Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
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PART I: CHANGES TO SWPPP

Does the corrective action based on this inspection require modification to the SWPPP? Yes No

Date of SWPPP update:

Brief description of the changes:

Action Initiated Date: Click here to enter a date. Initials:	Action Completed Date: Click here to enter a date. Initials
--	---

PART J: TRAINED INDIVIDUAL CERTIFICATION

I certify that Part A-I of this evaluation were evaluated by me as a trained individual. To the best of my knowledge and belief, the information documented in the report is true, accurate, and complete.

Evaluator Name and Title:

Signature and Date: _____

CERTIFICATE OF COMPLETION & COMPLIANCE

Name of project: _____

Address of premises on which land alteration was accomplished: _____

Inspection Date(s): _____ Stormwater Permit Number: _____

Relative to plans prepared by: _____ on _____
(date)

I hereby certify that:

1. I am familiar with drainage requirements applicable to such land alteration (as set forth in the Stormwater Management Ordinance of Tippecanoe County); and
2. I (or a person under my direct supervision) have personally inspected the completed work and examined the drainage permit and its conditions, as-built plans, and final drainage calculations consistent with as-built conditions performed pursuant to the above referenced drainage permit; and
3. To the best of my knowledge, information, and belief, such land alteration has been performed and completed in conformity with all such drainage requirements, except _____

Signature: _____

Date: _____

Typed or Printed Name: _____

Phone: (____) _____

(SEAL)

Business Address: _____

SURVEYOR

ENGINEER

(circle one)

Indiana Registration No. _____

TERMINATION INSPECTION CHECKLIST

(To Be Completed by Tippecanoe County)

1. Compliance Requirements

All construction sites shall undergo a final inspection by the Tippecanoe County Drainage Board representative prior to submittal of a Notice of Termination (NOT) to IDEM by the project owner to document the site is stabilized, temporary BMPs have been removed and no future land disturbance will occur within the permitted boundaries. A completed copy of this form confirming compliance with project termination requirements, signed by the Tippecanoe County Drainage Board representative, is required to be sent by the applicant to IDEM along with the NOT.

2. Inspection Details

Project Name:	
Address/Lot #:	
IDEM Permit No. ("INR" followed by 6 digits):	
Inspection Performed By:	Date:

3. Notice of Termination (NOT) Verification Inspection Items

ITEM	YES	NO	N/A
A. Have all earth disturbing activities been completed?	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
B. Are all soils stabilized as appropriate?	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
C. Are all drainageways stabilized with either vegetation, rip rap, or other armament?	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
D. Have all temporary erosion and sediment control measures been removed?	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
E. Has all construction waste, trash, and debris been removed from the site?	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
F. Has all construction equipment and material been removed from the site?	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
G. Are all the permanent BMPs free of sediment accumulation resulting from construction activities?	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>

4. Corrective Actions Required (If Applicable) If you answered "no" to any of the above questions, describe any corrective action which must be taken to remedy the problem and when the corrective actions are to be completed.

Attach additional sheet(s) if needed

5. Compliance Assessment

<input type="checkbox"/> DEFICIENT – Applicant must schedule a re-inspection by Tippecanoe County Drainage Board after deficiencies are addressed.
<input type="checkbox"/> COMPLIANT – Applicant must upload a signed copy to IDEM along with the IDEM NOT online submittal.

6. Certification and Signature

Inspector Name and Title:	Phone:
Inspector Signature:	Date:

Tippecanoe County Certified As-Built Plans Checklist

(to be completed by the applicant)

After completion of construction of the project and before final approval of the stormwater management plan, this checklist and a professionally prepared and certified “as-built” set of plans shall be submitted to the Tippecanoe County Drainage Board for review. Additionally, a digital copy of the “as-built” plans is required in a format approved by the Tippecanoe County Drainage Board.

Date: _____

Project Name: _____

General location: _____

Submitted by: _____

Y	N	N/A	Minimum data to include on the as-built set of plans
			Pipe size and pipe material
			Invert elevations
			Top rim elevations
			Pipe structure lengths
			BMP types, dimensions, and boundaries/easements
			“As-planted” plans for BMPs, as applicable
			Data and calculations showing that the as-built detention basin storage volume meets or exceeds the designed volume at each 1 foot interval above the basin normal pool
			Data and calculations showing BMP treatment capacity
			Side yard and rear yard swales flowline elevations and cross-sections every 100 feet
			The invert elevations, dimensions, and configurations of detention basin emergency overflow spillways
			Certified statements, by an independent certified geotechnical engineer experienced in the design, inspection, and construction of impoundment berms or dams, providing the soil characteristics, soil compaction, crest elevation, profiles, and the presence of any seepage control devices, if required, for any berm or dam constructed for the purpose of impounding water

			Riprap and paved side ditch location and elevations every 100 feet
			100-year flood emergency routing with flowline elevations and cross-sections every 100 feet
			Any other as-built drainage items as required by the Tippecanoe County Drainage Board
			Certified statement on plans stating that the completed storm drainage system and stormwater management facilities substantially comply with construction plans and the stormwater management permit as approved by the Tippecanoe County Drainage Board. (See Certification of Completion & Compliance form in Appendix B of the Technical Standards)

B3 - Individual Lot/Parcel Permit Forms

Individual Lot Stormwater Pollution Prevention Requirements
Individual Lot Certification Form

Individual Lot/Parcel Stormwater Pollution Prevention Requirements

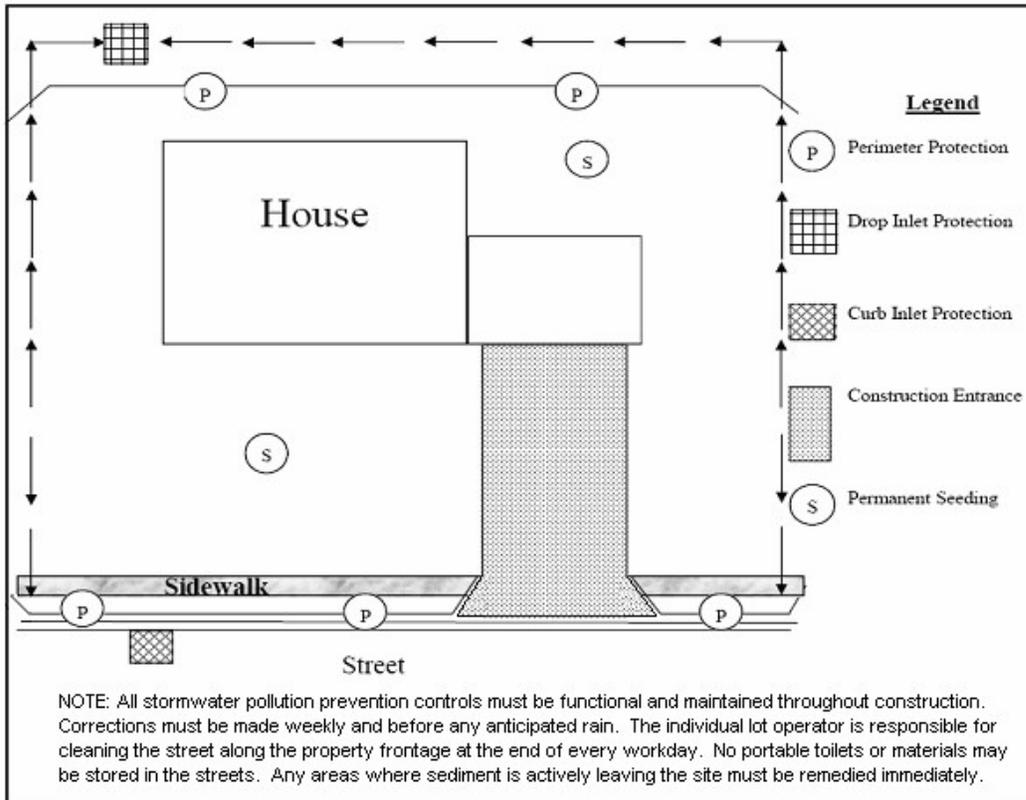
FOR COMPLIANCE WITH TIPPECANOE COUNTY DRAINAGE BOARD
STORMWATER CODE

THE INDIVIDUAL LOT OWNER OR OPERATOR IS RESPONSIBLE FOR THE INSTALLATION AND MAINTENANCE OF STORMWATER POLLUTION PREVENTION CONTROLS UNTIL THE ENTIRE LOT IS COMPLETE AND 100% STABILIZED.

- Initial Stormwater controls such as construction entrances, curbside and rear of lot storm inlet protection (geo-textile wrapped under grate is prohibited), and perimeter controls (typically silt fence) must be in place BEFORE ANY LOT DISTURBANCE begins.
- All construction materials must be staged off of the street and on the lot behind perimeter controls.
- Portable toilets must be kept off of the street and should be placed on even ground on the lot behind perimeter controls.
- All lots must provide and utilize appropriate trash containment for site waste.
- Contractors must use appropriate practices for concrete, mortar, and paint washout. These materials must be properly contained and NOT enter the storm drains or other conveyances.
- Any off site tracking of sediment into the street, or off site sedimentation into swales or drains MUST be cleaned as soon as possible and by no later than the end of the day.
- Lot frontage should be cleaned and acceptable in appearance at the end of every business day.
- Areas where operations have impacted adjacent lots or rear yard swales MUST be repaired to design condition and 100% stabilized.

PLEASE NOTE: ANY INSPECTION WILL FAIL IF THE CORRECT STORMWATER POLLUTION PREVENTION CONTROLS ARE NOT IN PLACE AND PROPERLY MAINTAINED.

Stormwater Pollution Prevention Controls on Individual Lots



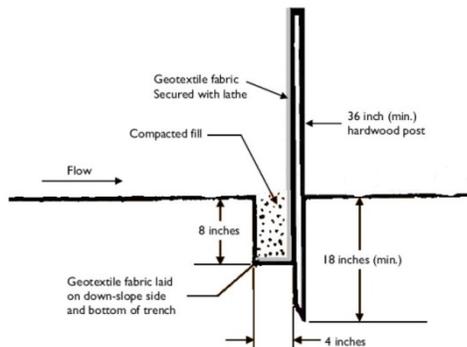
Construction Sequence

1. Install construction entrance.
 - a. Use #2 stone. Flare entrance at street so it can handle vehicle turn radius. See Detail.
2. Install perimeter protection.
 - a. Protection along the sides of the lot is only necessary if the adjacent lot is built out or if stormwater runoff will drain in that direction.
 - b. Make sure perimeter protections are turned into the lot where they terminate to create a ponding area. See above diagram.
 - c. Rear of lot perimeter protection should be installed to protect the rear yard swale.
3. Install protections on storm inlets at curbside and at rear of lot.
 - a. Geo-textile or "fabric" wrapped underneath the grate is **PROHIBITED**.
 - b. Make sure curbside inlet protection leaves the top 3-4 inches of the storm grate exposed to allow for overflow to enter the storm inlet, preventing ponding.
4. Lot disturbance may begin once the controls listed above are in place.
5. Maintain lot controls at all times and repair as soon as possible when a correction is needed.
 - a. If sediment is actively leaving the site due to a failing control such as tracking or an operation such as dewatering, it must be corrected **IMMEDIATELY**.
6. Stabilize all exposed soils with vegetation, mulch, or stone when construction is complete.
 - a. Lot is considered stabilized once vegetation has reached 100% coverage and 70 % density.
7. Remove temporary stormwater pollution prevention controls.
 - a. These may be removed when exposed soils have been stabilized with vegetation, sod, or mulch.
 - b. Seed alone is not a stabilization measure until it germinates and achieves proper coverage.

BMP DETAILS

Silt Fence

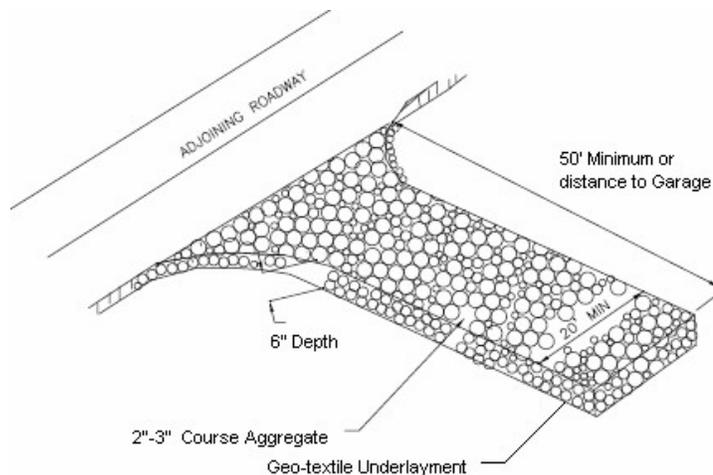
1. Install silt fence parallel to the contour of the land.
2. Extend ends of silt fence upslope 3-4 feet to allow for ponding areas behind the fence.
3. Excavate trench 8 inches deep and 4 inches wide.
4. Install with stakes on the down slope side of fence.
5. Bury 12 inches of fabric in the trench, extending the bottom 4 inches toward the upslope side.
6. Backfill trench on both sides of fence and compact.
7. Join silt fence sections by wrapping two posts and driving them in together. Do not use any other method of joining.



8. Inspect weekly and within 24 hrs of a ½" of rain. Silt fence should be cleaned out when the sediment has reached 1/3 the height of the exposed fencing. Repair silt fence where torn or damaged. Complete repairs before the next anticipated rain and by no later than one week from the date they are noticed.

Construction Entrance

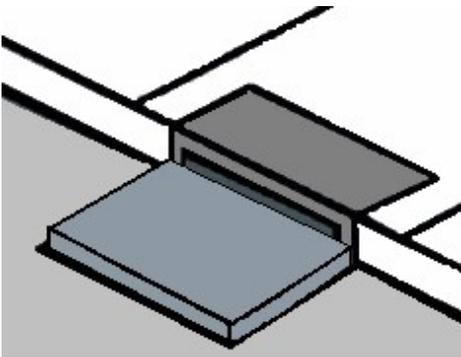
1. Install construction entrance from street to face of proposed building or at a 50' minimum length. Use #2 stone at a 6" minimum depth.
2. A geo-textile is required underneath the entrance to extend its functionality.
3. Flare out entrance where it meets the street so that vehicle turn radiuses do not travel over disturbed ground.



4. Perimeter Controls (silt fence) should be turned into the lot for a few feet where they meet the construction entrance.
5. Inspect weekly and within 24 hrs of a ½” of rain. Freshen or replace stone as needed to prevent off site tracking. If offsite tracking is occurring, clean up immediately, and correct the reason why the drive is failing as soon as possible. Complete repairs before the next anticipated rain and by no later than one week from the date they are noted.

Inlet Protection

1. Install inlet protection on all curbside and rear of lot storm
2. Curbside inlet protection should be installed so that 3 – 4 inches of the top of the casting is exposed to allow for overflow, preventing excessive ponding.
3. Wrapping geo-textile underneath the grate for protection or straw bale barriers are PROHIBITED practices.
4. Make sure inlet protection is securely fastened to the storm grate and installed per the manufacturer’s recommendations.



5. Inspect weekly and within 24 hours of a ½” of rain. Sediment accumulation or standing water around the inlet can indicate the need for maintenance. Clean protection when clogged with sediment or when it reaches ½ of the storage capacity or height of the control. Replace protection if torn or worn. Clean sediment from street around the storm inlet and place back onto lot behind perimeter controls. Complete repairs before the next anticipated rain and by no later than one week from the day they are noted.

APPENDIX C

CONSTRUCTION BMPs

BMP CN – 101 WHEEL WASH

DESCRIPTION

When a stabilized construction entrance is not preventing sediment from being tracked onto pavement, a wheel wash may be installed. Wheel washing is generally an effective BMP when installed with careful attention to topography. For example, a wheel wash can be detrimental if installed at the top of a slope abutting a right-of-way where the water from the dripping truck can run unimpeded into the street. Pressure washing combined with an adequately sized and surfaced pad with direct drainage to a large 10-foot x 10-foot sump can be very effective.

ADVANTAGES

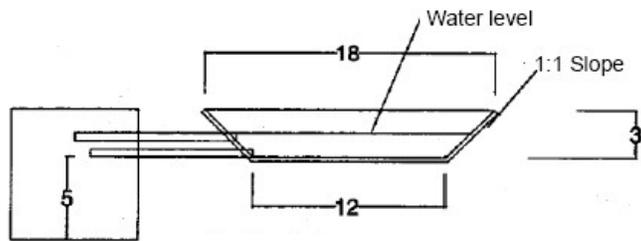
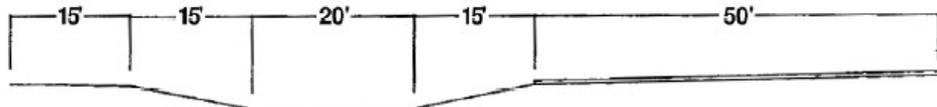
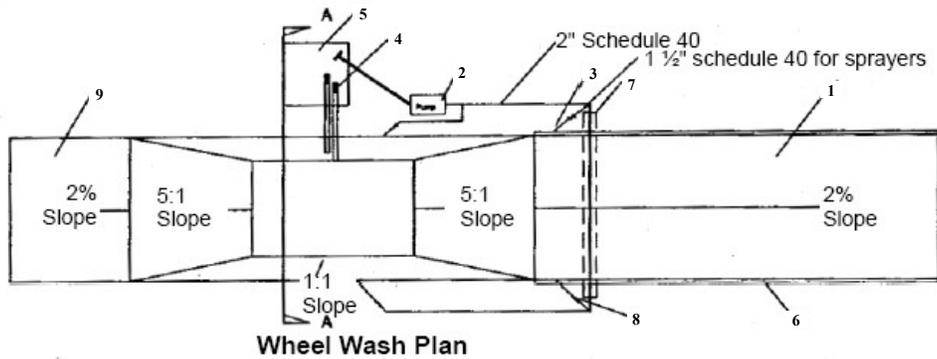
1. Wheel washes reduce the amount of sediment transported onto paved roads by motor vehicles.

DESIGN CRITERIA

1. Suggested details are shown in Figure CN-102-A. The Tippecanoe County Drainage Board may allow other designs.
2. A minimum of 6 inches of asphalt treated base (ATB) over crushed base material or 8 inches over a good subgrade is recommended to pave the wheel wash.
3. Use a low clearance truck to test the wheel wash before paving. Either a belly dump or lowboy will work well to test clearance.
4. Keep the water level from 12 to 14 inches deep to avoid damage to truck hubs and filling the truck tongues with water.
5. Midpoint spray nozzles are only needed in extremely muddy conditions.
6. Wheel wash systems should be designed with a small grade change, 6 to 12 inches for a 10-foot-wide pond, to allow sediment to flow to the low side of pond to help prevent re-suspension of sediment.
7. A drainpipe with a 2- to 3-foot riser should be installed on the low side of the pond to allow for easy cleaning and refilling.
8. Polymers may be used to promote coagulation and flocculation in a closed-loop system. Polyacrylamide (PAM) added to the wheel wash water at a rate of 0.25 - 0.5 pounds per 1,000 gallons of water increases effectiveness and reduces cleanup time.
9. If PAM is already being used for dust or erosion control and is being applied by a water truck, the same truck can be used to change the wash water.
10. The wheel wash should start out the day with fresh water. The wash water should be changed a minimum of once per day.
11. On large earthwork jobs where more than 10-20 trucks per hour are expected, the wash water will need to be changed more often.
12. Wheel wash or tire bath wastewater shall be discharged to a separate on-site treatment system, such as closed-loop recirculation or land application, or to the sanitary sewer with proper local sewer utility approval.

REFERENCE

City of Tacoma, Surface Water Management Manual, 2003 or later



Section A-A
NTS

- Notes:
1. Asphalt construction entrance 6 in. asphalt treated base (ATB).
 2. 3-inch trash pump with floats on the suction hose.
 3. Midpoint spray nozzles, if needed.
 4. 6-inch sewer pipe with butterfly valves. Bottom one is a drain. Locate top pipe's invert 1 foot above bottom of wheel wash.
 5. 8 foot x 8 foot sump with 5 feet of catch. Build so can be cleaned with trackhoe.
 6. Asphalt curb on the low road side to direct water back to pond.
 7. 6-inch sleeve under road.
 8. Ball valves.
 9. 15 foot. ATB apron to protect ground from splashing water.

Figure CN-102-A

BMP CN – 102 DEWATERING STRUCTURE

DESCRIPTION

Water which is pumped from a construction site usually contains a large amount of sediment. A dewatering structure is designed to remove the sediment before water is released off-site.

This practice includes several types of dewatering structures which have different applications dependent upon site conditions and types of operation. Other innovative techniques for accomplishing the same purpose are encouraged, but only after specific plans and details are submitted to and approved by the Tippecanoe County Drainage Board.

DESIGN CRITERIA

1. A dewatering structure must be sized (and operated) to allow pumped water to flow through the filtering device without overtopping the structure.
2. Material from any required excavation shall be stored in an area and protected in a manner that will prevent sediments from eroding and moving off-site.
3. An excavated basin (applicable to "Straw Bale/Silt Fence Pit") may be lined with filter fabric to help reduce scour and to prevent the inclusion of soil from within the structure.
4. Design criteria more specific to each particular dewatering device can be found in Figures CN-103-A through CN-103-C.
5. A dewatering structure may not be needed if there is a well-stabilized, vegetated area onsite to which water may be discharged. The area must be stabilized so that it can filter sediment and at the same time withstand the velocity of the discharged water without eroding. A minimum filtering length of 75 feet must be available in order for such a method to be feasible.
6. The filtering devices must be inspected frequently and repaired or replaced once the sediment build-up prevents the structure from functioning as designed.
7. The accumulated sediment which is removed from a dewatering device must be spread on-site and stabilized or disposed of at an approved disposal site as per approved plan.

Portable Sediment Tank (see Figure CN103-A)

- The structure may be constructed with steel drums, sturdy wood or other material suitable for handling the pressure exerted by the volume of water.
- Sediment tanks will have a minimum depth of 2 ft.
- The sediment tank shall be located for easy clean-out and disposal of the trapped sediment and to minimize the interference with construction activities.
- The following formula shall be used to determine the storage volume of the sediment tank:

$$\text{Pump discharge (gallons/min.)} \times 16 = \text{cubic feet of storage required}$$

- Once the water level nears the top of the tank, the pump must be shut off while the tank drains and additional capacity is made available.
- The tank shall be designed to allow for emergency flow over top of the tank. Clean-out of the tank is required once one-third of the original capacity is depleted due to sediment accumulation. The tank shall be clearly marked showing the clean-out point.

Filter Box (see Figure CN-103-B)

- The box selected should be made of steel, sturdy wood or other materials suitable to handle the pressure requirements imposed by the volume of water. Normally readily available 55-gallon drums welded top to bottom will suffice in most cases.
- Bottom of the box shall be made porous by drilling holes (or some other method).
- Coarse aggregate shall be placed over the holes at a minimum depth of 12 inches, metal “hardware” cloth may need to be placed between the aggregate and the holes if holes are drilled larger than the majority of the stone.
- As a result of the fast rate of flow of sediment-laden water through the aggregate, the effluent must be directed over a well-vegetated strip of at least 50 feet after leaving the base of the filter box.
- The box shall be sized as follows:

$$\text{Pump discharge (gallons/min.)} \times 16 = \text{cubic feet of storage required}$$

- Once the water level nears the top of the box, the pump must be shut off while the box drains and additional capacity is made available.
- The box shall be designed/constructed to allow for emergency flow over the top of this box.
- Clean-out of the box is required once one-third of the original capacity is depleted due to sediment accumulation. The tank shall be clearly marked showing the clean-out point.
- If the stone filter does become clogged with sediment so that it no longer adequately performs its function, the stones must be pulled away from the inlet, cleaned and replaced.
- Using a filter box only allows for minimal settling time for sediment particles; therefore, it should only be used when site conditions restrict the use of the other methods.

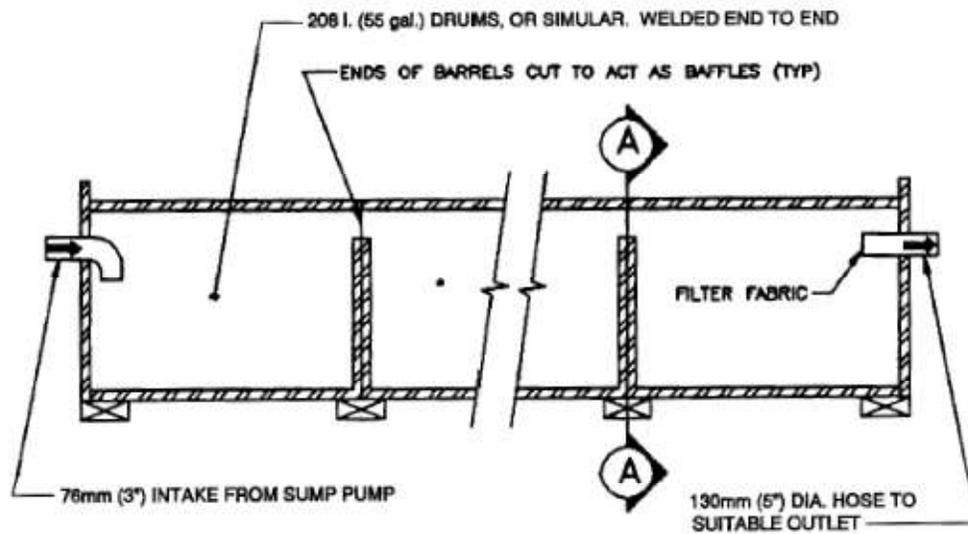
Straw Bale/Silt Fence Pit (see Figure CN-103-C)

- Measure shall consist of straw bales, silt fence, a stone outlet (a combination of riprap and aggregate) and a wet storage pit oriented as shown in Figure CN-103-C.
- The structure must have a capacity which is dictated by the following formula:
$$\text{Pump discharge (gallons/min.)} \times 16 = \text{cubic feet of storage required}$$
- In calculating the capacity, one should include the volume available from the floor of the excavation to the crest of the stone weir.
- In any case, the excavated area should be a minimum of 3 feet below the base of the perimeter measures (straw bales or silt fence).
- The perimeter measures must be installed as per the guidelines found in BMP-4, STRAW BALE BARRIER and BMP-5, SILT FENCE.
- Once the water level nears the crest of the stone weir (emergency overflow), the pump must be shut off while the structure drains down to the elevation of the wet storage.
- The wet storage pit may be dewatered only after a minimum of 6 hours of sediment settling time. This effluent should be pumped across a well vegetated area or through a silt fence prior to entering a watercourse.
- Once the wet storage area becomes filled to one-half of the, excavated depth, accumulated sediment shall be removed and properly disposed of.

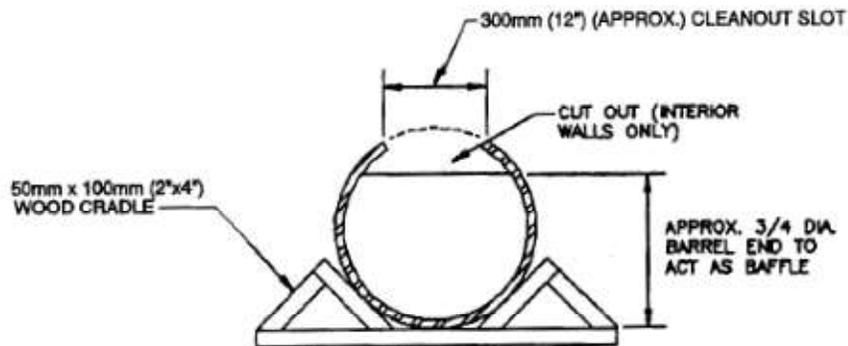
- Once the device has been removed, ground contours will be returned to original condition.

REFERENCE

United States Army Corps of Engineers, Handbook for the Preparation of Storm Water Pollution Prevention Plans for Construction Activities, 1997 or later

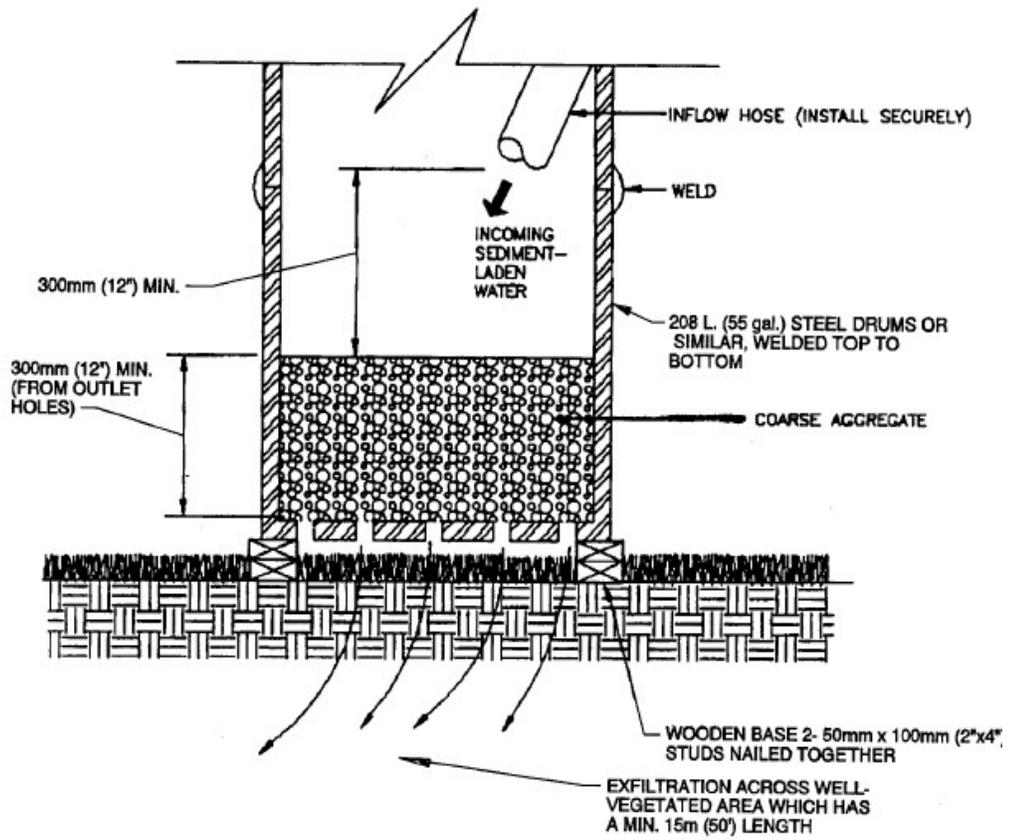


ELEVATION



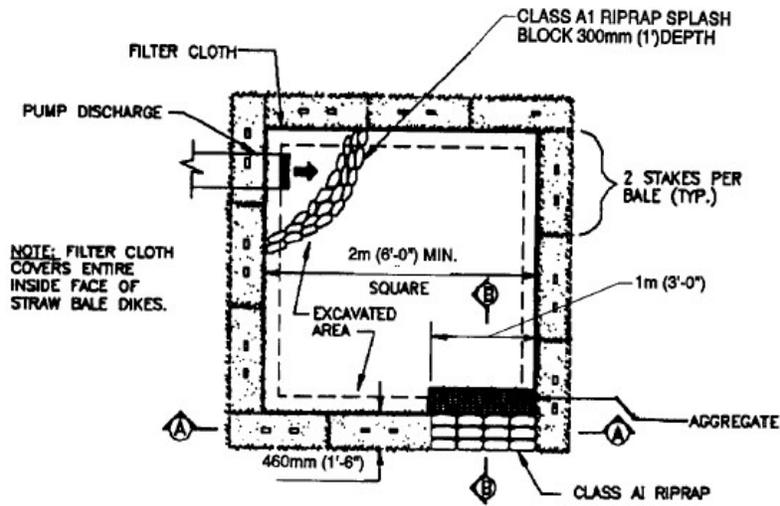
CROSS-SECTION A-A

**Figure CN-103-A
Portable Sediment Tank**

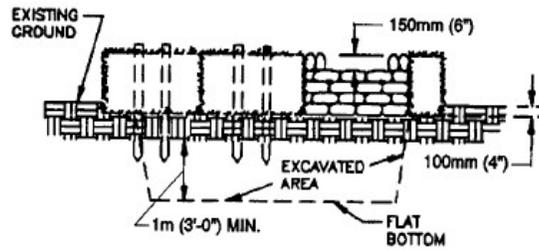


ELEVATION VIEW

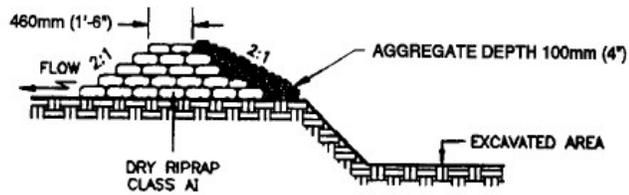
Figure CN-103-B
Filter Box



PLAN VIEW



CROSS-SECTION A-A



CROSS-SECTION B-B

Figure CN-103-C
Straw Bale/Silt Fence Pit

BMP CN – 103

SPILL PREVENTION AND CONTROL

DESCRIPTION

These procedures and practices are implemented to prevent and control spills in a manner that minimizes or prevents the discharge of spilled material to the drainage system or watercourses.

This best management practice (BMP) applies to all construction projects. Spill control procedures are implemented anytime chemicals and/or hazardous substances are stored. Substances may include, but are not limited to:

- Soil stabilizers/binders
- Dust Palliatives
- Herbicides
- Growth inhibitors
- Fertilizers
- Deicing/anti-icing chemicals
- Fuels
- Lubricants
- Other petroleum distillates

To the extent that the work can be accomplished safely, spills of oil, petroleum products, sanitary and septic wastes, and substances listed under 40 CFR parts 110, 117, and 302, and shall be contained and cleaned up immediately.

LIMITATIONS

1. This BMP only applies to spills caused by the contractor.
2. Procedures and practices presented in this BMP are general. Contractor shall identify appropriate practices for the specific materials used or stored on-site in advance of their arrival at the site.

DESIGN CRITERIA

1. To the extent that it doesn't compromise clean up activities, spills shall be covered and protected from stormwater runoff during rainfall.
2. Spills shall not be buried or washed with water.
3. Used clean up materials, contaminated materials, and recovered spill material that is no longer suitable for the intended purpose shall be stored and disposed of in conformance with BMP CN-106: Hazardous Waste Management.
4. Water used for cleaning and decontamination shall not be allowed to enter storm drains or watercourses and shall be collected and disposed of in accordance with BMP CN-106: Hazardous Waste Management.
5. Water overflow or minor water spillage shall be contained and shall not be allowed to discharge into drainage facilities or watercourses.

6. Proper storage, clean-up and spill reporting instruction for hazardous materials stored or used on the project site shall be posted at all times in an open, conspicuous and accessible location.
7. Waste storage areas shall be kept clean, well organized and equipped with ample clean-up supplies as appropriate for the materials being stored. Perimeter controls, containment structures, covers and liners shall be repaired or replaced as needed to maintain proper function.
8. Verify weekly that spill control and clean up materials are located near material storage, unloading, and use areas.
9. Update spill prevention and control plans and stock appropriate clean-up materials whenever changes occur in the types of chemicals used or stored onsite.

Cleanup and Storage Procedures for Minor Spills

- Minor spills typically involve small quantities of oil, gasoline, paint, etc., which can be controlled by the first responder at the discovery of the spill.
- Use absorbent materials on small spills rather than hosing down or burying the spill.
- Remove the absorbent materials promptly and dispose of properly.
- The practice commonly followed for a minor spill is:
 - Contain the spread of the spill.
 - Recover spilled materials.
 - Clean the contaminated area and/or properly dispose of contaminated materials.

Cleanup and Storage Procedures for Semi-Significant Spills

- Semi-significant spills still can be controlled by the first responder along with the aid of other personnel such as laborers and the foreman, etc. This response may require the cessation of all other activities.
- Clean up spills immediately:
- Notify the project foreman immediately. The foreman shall notify the **Tippecanoe County Emergency Management Agency's (TEMA) Hazardous Materials Response Team at (765) 742-1334.**
- Contain spread of the spill.
- If the spill occurs on paved or impermeable surfaces, clean up using "dry" methods (absorbent materials, cat litter and/or rags). Contain the spill by encircling with absorbent materials and do not let the spill spread widely.
- If the spill occurs in dirt areas, immediately contain the spill by constructing an earthen dike. Dig up and properly dispose of contaminated soil.
- If the spill occurs during rain, cover spill with tarps or other material to prevent contaminating runoff.

Cleanup and Storage Procedures for Significant/Hazardous Spills

- For significant or hazardous spills that cannot be controlled by personnel in the immediate vicinity, notify the local emergency response by dialing 911. In addition to 911, the contractor will notify the proper county officials. It is the contractor's responsibility to have all emergency phone numbers at the construction site.

- For spills of federal reportable quantities, in conformance with the requirements in 40 CFR parts 110,119, and 302, the contractor shall notify the National Response Center at (800) 424-8802.
- Notification shall first be made by telephone and followed up with a written report.
- The services of a spills contractor or a Haz-Mat team shall be obtained immediately. Construction personnel shall not attempt to clean up the spill until the appropriate and qualified personnel have arrived at the job site.

REFERENCE

California Department of Transportation, Construction Site BMP Manual, 2000 or later

BMP CN – 104

SOLID WASTE MANAGEMENT

DESCRIPTION

Solid waste management procedures and practices are designed to minimize or eliminate the discharge of pollutants to the drainage system or to watercourses as a result of the creation, stockpiling, or removal of construction site wastes.

Solid waste management procedures and practices are implemented on all construction projects that generate solid wastes.

Solid wastes include but are not limited to:

1. Construction wastes including brick, mortar, timber, steel and metal scraps, sawdust, pipe and electrical cuttings, non-hazardous equipment parts, styrofoam and other materials used to transport and package construction materials.
2. Landscaping wastes, including vegetative material, plant containers, and packaging materials.
3. Litter, including food containers, beverage cans, coffee cups, paper bags, plastic wrappers, and smoking materials, including litter generated by the public.

LIMITATIONS

1. Temporary stockpiling of certain construction wastes may not necessitate stringent drainage related controls during the non-rainy season.

DESIGN CRITERIA

1. Dumpsters of sufficient size and number shall be provided to contain the solid waste generated by the project and properly serviced.
2. Littering on the project site shall be prohibited.
3. To prevent clogging of the storm drainage system, litter and debris removal from drainage grates, trash racks, and ditch lines shall be a priority.
4. Trash receptacles with lids shall be provided in the Contractor's yard, field trailer areas, and at locations where workers congregate for lunch and break periods.
5. Construction debris and litter from work areas within the construction limits of the project site shall be collected and placed in watertight dumpsters at least weekly regardless of whether the litter was generated by the Contractor, the public, or others. Collected litter and debris shall not be placed in or next to drain inlets, storm water drainage systems or watercourses.
6. Full dumpsters shall be removed from the project site and the contents shall be disposed of, off-site, in an appropriate manner.;
7. Litter stored in collection areas and containers shall be handled and disposed of by trash hauling contractors.
8. Construction debris and waste shall be removed from the site every two weeks.
9. Stormwater run-off shall be prevented from contacting stored solid waste through the use of berms, dikes, or other temporary diversion structures or through the use of measures to elevate waste from site surfaces.
10. Solid waste storage areas shall be located at least 50 ft from drainage facilities and watercourses and shall not be located in areas prone to flooding or ponding.

11. Except during fair weather, construction and landscaping waste not stored in watertight dumpsters shall be securely covered from wind and rain by covering the waste with tarps, plastic sheeting, or equivalent.
12. Dumpster washout on the project site is not allowed.
13. Notify trash hauling contractors that only watertight dumpsters are acceptable for use on-site.
14. Plan for additional containers during the demolition phase of construction.
15. Plan for more frequent pickup during the demolition phase of construction.
16. Construction waste shall be stored in a designated area. Access to the designated area shall either be well vegetated ground, a concrete or asphalt road or drive, or a gravel construction entrance, to avoid mud tracking by trash hauling contractors.
17. Segregate potentially hazardous waste from non-hazardous construction site waste.
18. Keep the site clean of litter debris.
19. Make sure that toxic liquid wastes (e.g., used oils, solvents, and paints) and chemicals (e.g., acids, pesticides, additives, curing compounds) are not disposed of in dumpsters designated for construction debris.
20. For disposal of hazardous waste, see BMP CN-106: Hazardous Waste Management. Have hazardous waste hauled to an appropriate disposal and/or recycling facility.
21. Salvage or recycle useful vegetation debris, packaging and/or surplus building materials when practical. For example, trees and shrubs from land clearing can be converted into wood chips, then used as mulch on graded areas. Wood pallets, cardboard boxes, and construction scraps can also be recycled.
22. Prohibit littering by employees, subcontractors, and visitors.
23. Wherever possible, minimize production of solid waste materials.

REFERENCE

California Department of Transportation, Construction Site BMP Manual, 2000 or later

BMP CN – 105

HAZARDOUS WASTE MANAGEMENT

DESCRIPTION

These are procedures and practices to minimize or eliminate the discharge of pollutants from construction site hazardous waste to the storm drain systems or to watercourses.

This best management practice (BMP) applies to all construction projects.

Hazardous waste management practices are implemented on construction projects that generate waste from the use of:

- Petroleum Products,
- Asphalt Products,
- Concrete Curing Compounds,
- Pesticides,
- Acids,
- Paints,
- Stains,
- Solvents,
- Wood Preservatives,
- Roofing Tar, or
- Any materials deemed a hazardous waste in 40 CFR Parts 110, 117, 261, or 302.

DESIGN CRITERIA

Storage Procedures

1. Wastes shall be stored in sealed containers constructed of a suitable material and shall be labeled as required by 49 CFR Parts 172, 173, 178, and 179.
2. All hazardous waste shall be stored, transported, and disposed as required in 49 CFR 261-263.
3. Waste containers shall be stored in temporary containment facilities that shall comply with the following requirements:
 - Temporary containment facility shall provide for a spill containment volume able to contain precipitation from a 24-hour, 25 year storm event, plus the greater of 10% of the aggregate volume of all containers or 100% of the capacity of the largest tank within its boundary, whichever is greater.
 - Temporary containment facility shall be impervious to the materials stored there for a minimum contact time of 72 hours.
 - Temporary containment facilities shall be maintained free of accumulated rainwater and spills. In the event of spills or leaks accumulated rainwater and spills shall be placed into drums after each rainfall. These liquids shall be handled as a hazardous waste unless testing determines them to be non-hazardous. Non-hazardous liquids shall be sent to an approved disposal site.
 - Sufficient separation shall be provided between stored containers to allow for spill cleanup and emergency response access.
 - Incompatible materials, such as chlorine and ammonia, shall not be stored in the same temporary containment facility.

- Throughout the rainy season, temporary containment facilities shall be covered during non-working days, and prior to rain events. Covered facilities may include use of plastic tarps for small facilities or constructed roofs with overhangs. A storage facility having a solid cover and sides is preferred to a temporary tarp. Storage facilities shall be equipped with adequate ventilation.
- 4. Drums shall not be overfilled and wastes shall not be mixed.
- 5. Unless watertight, containers of dry waste shall be stored on pallets.
- 6. Paint brushes and equipment for water and oil based paints shall be cleaned within a contained area and shall not be allowed to contaminate site soils, watercourses or drainage systems. Waste paints, thinners, solvents, residues, and sludges that cannot be recycled or reused shall be disposed of as hazardous waste. When thoroughly dry, latex paint and paint cans, used brushes, rags, absorbent materials, and drop cloths shall be disposed of as solid waste.
- 7. Ensure that adequate hazardous waste storage volume is available.
- 8. Ensure that hazardous waste collection containers are conveniently located.
- 9. Designate hazardous waste storage areas on site away from storm drains or watercourses and away from moving vehicles and equipment to prevent accidental spills.
- 10. Minimize production or generation of hazardous materials and hazardous waste on the job site.
- 11. Use containment berms in fueling and maintenance areas and where the potential for spills is high.
- 12. Segregate potentially hazardous waste from non-hazardous construction site debris.
- 13. Keep liquid or semi-liquid hazardous waste in appropriate containers (closed drums or similar) and under cover.
- 14. Clearly label all hazardous waste containers with the waste being stored and the date of accumulation.
- 15. Place hazardous waste containers in secondary containment.
- 16. Do not allow potentially hazardous waste materials to accumulate on the ground.
- 17. Do not mix wastes.

Disposal Procedures

1. Waste shall be removed from the site within 90 days of being generated.
2. Waste shall be disposed of by a licensed hazardous waste transporter at an authorized and licensed disposal facility or recycling facility utilizing properly completed Uniform Hazardous Waste Manifest forms.
3. A certified laboratory shall sample waste and classify it to determine the appropriate disposal facility.
4. Make sure that toxic liquid wastes (e.g., used oils, solvents, and paints) and chemicals (e.g., acids, pesticides, additives, curing compounds) are not disposed of in dumpsters designated for solid waste construction debris.
5. Properly dispose of rainwater in secondary containment that may have mixed with hazardous waste.
6. Recycle any useful material such as used oil or water-based paint when practical.

Maintenance and Inspection

1. A foreman and/or construction supervisor shall monitor on-site hazardous waste storage and disposal procedures.
2. Waste storage areas shall be kept clean, well organized, and equipped with ample clean-up supplies as appropriate for the materials being stored.

3. Storage areas shall be inspected in conformance with the provisions in the contract documents.
4. Perimeter controls, containment structures, covers, and liners shall be repaired or replaced as needed to maintain proper function.
5. Hazardous spills shall be cleaned up and reported in conformance with the applicable Material Safety Data Sheet (MSDS) and the instructions posted at the project site.
6. The National Response Center, at (800) 424-8802, shall be notified of spills of Federal reportable quantities in conformance with the requirements in 40 CFR parts 110, 117, and 302.
7. Copy of the hazardous waste manifests shall be provided to the Owner.

REFERENCE

California Department of Transportation, Construction Site BMP Manual, 2000 or later